Annex O

Dam Safety Plan for the BGHES

Zambezi River Authority

BATOKA GORGE HYDRO-ELECTRIC SCHEME

Phase III - FEASIBILITY

Vol.7 - DAM SAFETY PLAN

May 2019

Number													
3	4	6	G	E	N	R	S	P	0	0	1	B	May 2019
section			subsection			type	office		serial n°			rev	date

studio pietrangeli
consulting engineers

LIST OF VOLUMES

Vol.1 – 300 GEN R SP 002 – MAIN REPORT

Vol.2 – 300 GEN D SP 001 – FEASIBILITY DESIGN DRAWINGS

Vol.3 – 230 GEN R SP 001 – LIDAR TOPOGRAPHIC SURVEY REPORT

Vol.4 – 310 HYD R SP 001 – HYDROLOGY AND CLIMATE CHANGE REPORT

Vol.5 – 320 GEN R SP 001 – GEOTECHNICAL BASELINE REPORT

Vol.6 – 320 SEI R SP 001 – SEISMIC HAZARD ASSESSMENT REPORT

Vol.7 – 346 GEN R SP 001 – DAM SAFETY PLAN

Vol.8 – 350 STA R SP 001 – DAM STRUCTURAL ANALYSIS REPORT

Vol.9 – 355 GEN R SP 001 A, SPILLWAY NUMERICAL MODEL 1ST RUN

Vol.10 – 375 GEN R SP 001 – RESERVOIR OPERATION AND ENERGY PRODUCTION STUDIES

Vol.11 – 380 GEN R SP 001 – ACCESS ROADS AND CAMPS

Vol.12 – 390 GEN R SP 001 – TRANSMISSION SYSTEM DESIGN REPORT

Vol.13 – 390 GEN R SP 002 – POWER EVACUATION STUDIES REPORT

CONTENTS

GENERAL INTRODUCTION

1.1 CONTENT AND STRUCTURE OF THE DAM SAFETY PLAN

The Dam Safety Plan is articulated in the following parts:

A. CONSTRUCTION SUPERVISION AND QUALITY CONTROL PLAN

This part contains the organization, procedures and activities required for supervision of the construction of the Batoka Plant.

B. INSTRUMENTATION PLAN

It describes the monitoring system of Batoka Dam, and it contains the instruction for measurements to be collected by the instruments, for their presentation, use and assessment.

C. OPERATIONAL PLAN (Preliminary Plan)

It provides the guidelines for the operation of the Batoka scheme, including the dam, power waterways, power house and other appurtenant structures.

D. MAINTENANCE PLAN (Preliminary Plan)

It outlines the operation and maintenance activities and procedures relevant to the Batoka Dam and Hydro Power Plants.

E. EMERGENCY PREPAREDNESS PLAN (FRAMEWORK PLAN)

It contains the Batoka Emergency Preparedness Plan that includes the description of types of emergencies and how to identify them, the actions to take in case of emergency and the preparedness and Emergency Response. Moreover, the dam break analysis is included in this part.

The Dam Safety Plan is prepared according to the World Bank Dam Safety safeguard policy (OP/BP 4.37).

This revision B has been updated in order to add the results of the dam break analysis in part. E.

Each part is self-standing. The dam safety plan is provided on the basis of the knowledge of the project at this feasibility design stage, and shall therefore be integrated and detailed with the development of the project, to become an operative tool for the operation of the dam and of the plants.

It is assumed that:

- The Dam Safety Plan, and in particular the operation and maintenance of the Dam and of the two Power Plants, will be assigned by the Plant Owners to their departments or entities or internal/external consultants dedicated to the control, operation and maintenance of dam and national power plants, that in this report will be always referred as "Plant Management Structure" (PMS).
- The project at this stage is considered as a whole, and this document is conceived as a base from which it will be possible:
    - To split it in lots, and to identify possible limits and definition of responsibility among the actors involved in the owning and management of the dam and plants.

  - to develop in detail the Dam Safety Plan, progressively with the subsequent developments of the project, to become an operative tool for the operation of the dam and of the plants.

- This report is based on the Feasibility Design of Batoka Project, to which reference is made. This report is deemed therefore integrated by the Feasibility Design reports and drawings.

  It is assumed in particular that this plan will be integrated progressively with:

  - detailed design drawings and reports
  - then the as-built drawings
  - original equipment or materials (as far as applicable) manufacturers' operations and maintenance recommendations and manuals,
  - EM and HSS Equipment Operation and Maintenance Manuals.

- This plan must be reviewed and updated as necessary:
  - To reflect the conditions of the detailed design and then of the construction on site whenever they will result substantially different from what assumed in this report.
  - Whenever any significant change to the scheme occurs, including any changes to the operating rules.
  - At intervals of 5 years after Plant commissioning.
  - Following any ownership change.

1.2 GLOSSARY AND ABBREVIATIONS

CDP	Controlled Drawdown Procedure
EAP	Emergency Action Plan
EDP	Emergency Drawdown Procedure
EM eq	Electro-mechanical equipment
EPP	Emergency Preparedness Plan
GS	Gate Shaft
HPP	Hydro Power Plant
HSS eq	Hydraulic Steel Structure equipment
IL	Invert level
MLO (or MO)	Middle Level Outlet (or Middle Outlet)
OHL	Over Head Line (electric transmission high voltage line)
PMS	Plant Management Structure
PH	Power House
PP	Plunge Pool
PT	Power Tunnel
RCC	Rolled Compacted Concrete
SS	Surge Shaft
SWY	Switchyard

SP	Studio Ing. G. Pietrangeli - Rome
ZRA	Zambesi River Authority

PART A - CONSTRUCTION SUPERVISION and QUALITY CONTROL PLAN

A.1 INTRODUCTION

A.1.1 CONTENT AND STRUCTURE OF THIS PART

This is the PART A "CONSTRUCTION SUPERVISION and QUALITY CONTROL PLAN" of the Batoka Dam Safety Plan.

This part contains the organization, procedures and activities required for supervision of the construction of the Batoka Plant.

Some of the guidelines provided in this section shall be detailed when the relevant construction contract(s) is put in place.

A.2 METHODOLOGY

A.2.1 INTRODUCTION

This chapter provides the description of the organization, staffing levels, procedures, equipment, and qualifications for supervision of the construction of Batoka dam.

It is developed at feasibility design stage, being assumed to be fine-tuned at the moment of these tasks assignment.

It is focused on the activities to be carried out for the Design Review, Construction Management, Supervision, Testing and Commissioning of Batoka Project.

It is conceived assuming that the works will be executed by the relevant Contractor(s) through EPC procurement method, therefore the duties of supervision will be in compliance with the requirements of the international practice foreseen in the "SILVER Book", prepared by the International Federation of Consulting Engineers (FIDIC) for this type of Contract.

Any adjustments in respect to the above assumptions are still possible in the further steps of design and construction phases, when also the structure that the Owner will put in place for the supervision (that for simplicity will be called hereinafter "Supervisor", that can be within his internal resources or recurring to external consultancy) will be defined.

A.2.2 CONSTRUCTION SUPERVISION AND QUALITY CONTROL PLAN ACTIVITIES

The Construction Supervision and quality control include the classical activities for the supervision of construction such as:

- DESIGN REVIEW

  Review and approval of the Contractor's design following the design implementation schedule in order to ensure technical soundness and compliance with the Client's requirements, prior to commencement of the Works.

  Design's review includes Feasibility Design appraisal, Draft Final and Final Design prepared by the Contractor for all Civil Works, Electro-mechanical equipment and Hydraulic Steel structures.
- REVIEW of TECHNICAL SPECIFICATIONS and BILL of QUANTITIES

  Review and approval of Technical Specifications and Bill of Quantities for all Civil Works, Electro-mechanical equipment and Hydraulic Steel structures to ensure that they fully comply with the Client's Requirements being economically sound.
- REVIEW of WORK PROGRAMME

  Review of Contractor's Work programme in order to verify its compliance with the Implementation Plan and to verify progress of the Works, actual and planned rate of production and highlight any criticality and/or delay emerged during the implementation of Design / Works.

- REVIEW of METHOD STATEMENTS
  Review of Contractor's Method Statements for all Civil Works, Electromechanical Works, Electrical Works and Hydraulic Steel Works, in order to ensure technical soundness of the proposed solutions.
- REVIEW of MANAGEMENT PLANS
  Review of the Management Plans of all the elements / personnel / equipment involved in the production chain of the Works to ensure timely production and quality of the Works.
- SUPERVISION during CONSTRUCTION, ERECTION TESTING and COMMISSIONING
  Supervision of construction, erection, testing and commissioning of all project Works to ensure their compliance with the approved Designs, Drawings, Specifications, Conditions of Contract, Work Programme and state-of-the-art Engineering Practice.
  During this phase of the assignment, it will be carried out a supervision and control of all environmental activities, including but not limited to those related to quarry exploitation, dumping of materials, impounding process and demobilization stages.
- MANAGEMENT of INVOICES and PAYMENTS
  Verification of all payment invoices issued by the Contractor and preparation of acceptance certificates for the Works completed.
- ADMINISTRATION of CLAIMS arising from the Contractor.
- POST-CONSTRUCTION SERVICES during defects liability period.
  Verification that the performances of all the elements of the project comply with the Contract, Technical Specifications and Client's Requirements.

The activities listed above, and detailed in the following paragraphs, shall be carried out throughout the entire Contract period, including Defect Liability Period, under and in close cooperation with the staff of the Client.
The activities shall be carried out in order to guarantee a smooth implementation of the works in accordance with the EPC contract, Client's Requirements and state-of-the art technology. For this purpose periodical and also specific Design / Managerial Coordination meetings are to be organized.

A.2.3 LEVEL 1 DESIGN REVIEW

In compliance with the condition of Contract and Client's Requirements, the Contractor will prepare and submit the Level 1 design of the civil, electrical and mechanical works. It is stressed that in this phase of the Design the Contractor shall start all the investigation activities required to finalize the Design of the Works.

Therefore, before starting construction activities, the Supervisor will review the investigation programme proposed by the Contractor and the Level 1 design, carrying out the following main activities:

- review the design in accordance with the Client Requirements, condition of Contract, state-of-the art engineering international practice;

- review the design in accordance with the actual site requirements and the results of latest investigations available;
- ensure that the most cost effective and technically sound alternatives are examined and appropriate designs are produced accordingly;
- examine thoroughly the technical documentation (bill of quantities, specifications and methods of measurement etc.) in order to point out any discrepancy or mistake which may jeopardise the prompt implementation of the project causing major changes, disputes and claims.

The Supervisor will verify that the Level 1 Design includes all the elements required for a full understanding by the Client or the Clients Representative of the key features proposed, such as:

- general arrangement and emplacement of the project structures;
- general hydraulic design of:
  - diversion structures;
  - main gated spillway;
  - Middle Level Outlets;
  - Ecological Discharge devices;
  - power waterways (including head losses calculations and basic transient analyses);
- design of electrical equipment;
- design of civil works;
- design of the mechanical equipment;
- design of the hydraulic steel structures;
- sizing and general characteristics of the main equipment of each powerhouse;
- drawings of the turbine parts and of the discharge pit and tailrace dimensioned at Level 1 stage;
- calculations verifying the average annual energy production;
- single line diagrams;
- Level 1 arrangement and assembly drawings of the powerhouse complex, showing construction of main components, leading dimensions and masses and confirmed locations;
- Level 1 study of the equipment erection sequence and requirements, to the extent needed to determine space and facilities required in the powerhouse erection areas;
- calculations and studies required to justify and support the proposed technical solutions. They will also include:
  - results of the field and laboratory investigations performed up to that moment and program for any additional investigations deemed necessary;
  - design criteria, standards and design codes proposed for the development of the designs during construction;
  - technical specifications for the civil and building works;
  - technical specifications for the hydraulic steel structures;

- technical specifications for the electrical works, including generating equipment and powerhouse electrics;
- technical specifications for the powerhouse lighting, ventilation, air conditioning, and fire-fighting systems;
- Quality Assurance Plan and manuals.
- a detailed program of the phases of design, material supply, manufacture, delivery, erection and commissioning of the equipment;
- any other technical documents to be submitted according to the Contract.

A "No Objection" will be issued when the submitted documents are correct and complete. "Verified-Comply with Comment" will indicate that the calculations and/or the main arrangements defined by the documents are considered essentially correct, but require minor rectifications so as to complete or clarify certain points having only trivial influence on the design, the behaviour and/or the safety of the structure or of the equipment. On the contrary, should the Supervisor consider the documentation insufficient, or as giving inadequate guarantee on the behaviour and/or the safety of the structure or equipment, the document will be classified as "Rejected, comply with comments, Re-submittal required". Such labelling will be transmitted together with the necessary explanations to correct the document so that the Contractor may re-submit the rectified documents.

A.2.4 CONTRACTOR'S COORDINATION ACTIVITIES

The Contractor will be requested to submit a clear procedure, to be approved by the Supervisor, which specifics the detailed design process of both the civil engineering structures and EM and HSS equipment. The main aim of this procedure is to guarantee that the design of these elements and also the erection process is carried out consistently and with the required coordination:

- reducing the problems of construction interfaces;
- guaranteeing the timely delivery on site of the necessary information for construction (construction drawings, method statements, installation manuals);
- assuring the availability on site of the required equipment, man-power and permanent materials.

The everyday follow-up of these procedures will be the responsibility of the Contractor who will report to the Supervisor.

A.2.5 LEVEL 2 CONSTRUCTION DESIGN REVIEW

The Supervisor will review the construction drawings and reports of the civil works, of the hydro-mechanical, mechanical and electrical equipment.

The review performed at this stage is aimed to verify that the construction and manufacturing design documents correspond to the design solutions and relevant comments formulated during the previous phase. In this regard the Contractor will submit for review the general arrangement drawings and relevant specifications and calculations of all components parts of the Works, i.e. all the documents required by the Contractor to construct, erect, test and commission all the components of the Permanent Works (construction drawings, detailed technical specifications, construction and erection method statements, commissioning and testing procedures, etc.).
The Contractor will prepare a list and a time schedule for preparation for the submission of the Construction and Manufacturing Design documents.

The design activity during construction includes the progressive preparation of "As-built" drawings, which are an essential part of the documentation needed to properly maintain all the works and the equipment in their lifetime and to eventually rehabilitate them.
The Contractor will be required to prepare and keep a complete, up-to-date, set of as-built drawings showing the locations, sizes and details of the works as executed, adopting the format, referencing system and other relevant details previously agreed.
The Supervisor will verify that all modifications of the civil, hydro-mechanical, electro-mechanical and electrical drawings, introduced after the acceptance of the drawing, have been correctly and properly recorded and that the drawings are complete in all parts and comply with the latest accepted version of the technical calculations.

A.2.5.1 Hydro and Electro-mechanical equipment

Before proceeding with the manufacturing of the:

- gates, linings, and other steel structures;
- mechanical, hydro-mechanical and electrical equipment

The Contractor will be required to submit the following documents for review and comment on conformity with design criteria by the Supervisor:

- calculations carried out for determining the plate thicknesses, the welding and other structural characteristics of the penstocks, clearly indicating the principles on which calculations are based;
- general assembly drawings, sufficient sub-assembly drawings and details to demonstrate that all parts will conform to the provisions and intent of the Contract and to the requirements of their installation and maintenance. These drawings will show all necessary dimensions and sub-assemblies in which the Contractor proposes to ship the steel elements. The Contractor will also submit grounding location and details installation drawings;
- other detailed design drawings and documents that the Supervisor may require.

The Contractor will submit all erection drawings, showing the sequence of EM and HSS erection, erection equipment required, field welding works, erection / welding / concrete embedment sequence, etc. for review.

A.2.5.2 Civil Works and Ancillary Installations

The Contractor will develop the detailed design for construction and manufacturing of the civil works and ancillary installations basing on the comments relevant to previous Design phase.

Submissions to the Supervisor for its review and comment related to the Design of civil works typically include:

- Level 2 drawing of the outdoor and underground excavations, together with the required rock support and slopes stabilisation measures, supported by the necessary geotechnical analyses;
- Level 2 drawings and calculations of RCC dam, including stability analysis, thermal analysis, dam zoning, instrumentation system;
- Level 2 concrete outline drawings, showing all dimensions, first and second stage concrete, contraction and construction joints, classes of concrete and finishing, etc.;
- concrete reinforcement drawings, showing bars diameters, and spacing, supported by the necessary structural analyses and by the equipment design data which may be necessary to check the loads and design criteria applied;
- arrangement and details of embedded parts (water stops, piping systems, conduits, embedded parts for equipment erection etc.);
- architectural and miscellaneous finishing drawings;
- Level 2 method statements for all civil construction activities;
- details of the installations (crushing plant, batching plant, cooling system, conveyor system, etc…);
- other Level 2 design drawings and documents that the Supervisor may require.

Notwithstanding any Supervisor review and comment, in the frame of the EPC Contract nature the Contractor will be fully responsible for the correctness of the detailed drawings of the civil works, particularly in relation to the loads, dimensions, embedded parts and any other interface with the equipment.

The Contractor will submit the Level 2 Design of the civil structures in an orderly sequence, following the priorities of the construction program, so that the time allocated for the Supervisor's comment and review and possible request for corrections and/or modifications will not cause delays, and excessive simultaneous submissions are avoided.

Any other drawing, calculation note and design document that the Contractor may deem necessary for his own use, such as reinforcing bars bending schedules, materials lists, etc. will be submitted to the Supervisor for its information.

A.2.5.3 Modification to Design

In case, during the implementation of the Works, the Contractor will propose modifications to the Design due to practical reasons or to account to un-expected local conditions or constrains, the Supervisor will verify and check:

- the need of such modifications,
- their suitability,
- the overall impacts on the Works in terms of time for completion, quality, safety, interferences and/or repercussions with other parts or subsequent phases of the project.

If the Supervisor finds that issuance of a variations would be essential and / or unavoidable (unless an emergency occurs affecting safety of the Works, or any delay with the variation shall give rise to a substantial time overrun) the Supervisor will provide a Level 1 report outlining the evaluation of such variation, including but not limited to the following:

- the Supervisor's opinion on the extent, if any, of the applicability of the varied Works compared to the approved Design;
- conformity to Client's Requirements and international state-of-the-art engineering practice;
- potential interferences and/or repercussions with other parts or phases of the Project;
- potential impacts on Project Implementation Plan.

It will be arranged in the shortest possible time coordination meetings with the parties, in order to examine the proposal for modification and review / incorporate the modification in the detailed Contractor's Work Program. The Supervisor will verify that the approved modification to Design is reflected and incorporated in all relevant Design documents including Construction drawings, Method Statements and Quality plans. The above process will be carried out, as far as possible, in order not to disrupt the Construction Schedule.

A.2.5.4 Structural Analysis

The Supervisor will ensure that the Contractor has carried out all the Level 2 calculations by applying the principles, rules, norms, and procedures discussed and verified beforehand by the Supervisor. The Supervisor will verify and approve detailed calculations, drawings and/or supporting reports submitted by the Contractor.

Structural analysis shall include all the main structures including but not limited to: RCC dam, embankments, powerhouse, diversion works, power tunnels, intake structures, shafts, switchyards, and all the ancillary works of the project.

A.2.6 RESPONSIBILITY OF CONSULTANT'S TEAM

Design review, Supervision of manufacturing, testing, erection, commission and Construction Management of all the project elements will be the responsibility the Supervisor.
To achieve these goals the Supervisor will mobilize and maintain on site throughout the entire construction and commissioning period the supervisory team, composed by Resident Engineers, Key Staff and support Staff, as detailed in following paragraphs.

Whenever the Supervisor duty will be assigned by the Owner (intended as final Client of the EPC Contract) to an external international Consultant, typically the Consultant's team will be structured so to be assisted by a Client's Project Team designated by the Client, with the main goal to use the opportunity of the Design Review and Construction Management of Batoka Project to transmit the international experience of the Consultant to Client's Project Team.
The Client's Project Team will assist the Consultant's Team in the field; however, the activity of Client's staff will not relieve the Consultant from any obligation and/or responsibility foreseen in his contract for services.
Typical structure and methodology for such Knowledge Transfer are given in relevant paragraphs.

A.2.7 QUALITY ASSURANCE AND SCHEDULE OF EXECUTED WORKS

The Contractor will be required to guarantee the continuous control over the quality of the Works and its schedule through a Quality Assurance Plan (QAP). This QAP should define, in particular:

- organization of Contractor's Quality System;
- Quality Plan for Design;
- materials that he proposes to incorporate into the construction;
- methods planned to carry out every part of the works, and the resources needed;
- programme of the tests and controls (with relevant records) he intends to follow to prove that the requested quality is obtained;
- schedule of works and control of deviations and relevant countermeasures;
- management of non-conformities.

The Contractor under the EPC Contract will be required to be fully responsible of the quality of the proposed design and construction. The primary duty of the Supervision regarding quality are to make sure that the Contractor's Quality Plan is working properly.

The Supervisor will therefore review the Quality Assurance Plan, perform inspections in the workshops and laboratories and work areas to assess the application and conformity to the Quality Assurance Plan.

For each Work phase the Supervisor's staff, while verifying that the relevant Construction documentations are made available and that construction equipment is mobilized by the Contractor in accordance with the Work programme, shall pay particularly attention while checking the following aspects:

- Organization and conditions of the Laboratory, including the competence of the Laboratory Staff.
- Method Statements proposed by the Contractor.
- Main equipment and materials should be in accordance with the Specifications and suitable to the works to be carried out.

  The Contractor must submit, for each material and equipment that he proposes to incorporate in the construction, a full documentation to prove that it fulfils the requirements of the Technical Specifications.
- Control plan that the Contractor intends to implement, which should meet the Contract specifications and assure that quality is obtained.
- Number and Location of Hold Points (which needs a mandatory inspection before proceeding with the works) and Witness points (required by the Parties to verify, inspect and witness the process of work).

A.2.8 REVIEW OF TOPOGRAPHY AND SURVEYING

The Supervisor shall review and verify all methods and procedures, as well as the man-power, equipment and organization of the Contractor for carrying out topography and surveying Works. The Consultant shall request for verifications and cross checks, to be carried out in his presence, as deemed necessary.

The Supervisor shall carry out independently, as far as necessary, topography and surveying Works aimed to:

- Verify the local geodetic survey network;
- Dimensional control of the already built structures and setting out points;
- Document the progress of the works, their compliance with Construction Drawings and the quantities reported by the Contractor.
- Mapping of the underground works and excavations slopes.

The above tasks can be performed using modern technologies (like drones and laser scanning instruments).

The use of Drone, if possible, allows to acquire in short time a photo-mosaics of the entire surveyed area and a 3-dimensional model that can be used to generate contours map, slope maps, etc.... to be used for design, quantities calculation, documentation of work progress etc.

A.2.9 SITE SUPERVISION

The Supervisor will be required to mobilize and maintain on site throughout the entire construction and commissioning period the supervisory team, composed by Resident Engineer, Key Staff and support Staff. Before the starting of site activities (supposedly for a period of few months after the signing of the contract) the Project manager and the Key staff (in charge of Design Review coordination activities) will start the review of Contractor's Design as well as of all the Level 1 documentation to be provided by the Contractor (including Procedures, Programme, Quality and Management Plans) at home office.

The Supervisor shall define his Organization Structure, personnel's qualifications and specific assignments both at the Project Site and at the Home Office.
Whenever the Supervisor duty will be assigned by the Owner (intended as final Client of the EPC Contract) to an external international Consultant, typically the Consultant Organization Structure shall include staff supplied by the Client (Client Project Team).
The proposed general organization of the Supervisor for the EPC Contract Management should clearly reflect and show the relationship and communication flow between the Supervisor's Project Coordination Office and the EPC Contractor structure.

Supervisor's team shall supervise permanently all construction, hydro-mechanical and electrical equipment erection, testing, and commissioning operations in order to ensure the conformity of the Project and its portions to the Client's Requirements, Contract Conditions, Quality standards, applicable norms within the Time-Schedule foreseen in the Contract documents.

In addition, the Supervisor, through his technical personnel, shall carry out, whenever deemed necessary, in accordance with the progress of work, inspection visits to the site as well as to all Workshops and installations of hydro-mechanical and electrical equipment where Project components are manufactured, assembled, tested and /or prepared for shipment.
The Project Manager/Resident Engineer, with his team, will draft periodical Progress Reports and will develop a project administration system. Regular meeting will be scheduled with the staff of the Client and the Contractor. Reconciliation and grievance procedures will be established and finally all activities of backstopping and invoicing will be managed since the beginning of the project.

At the early stage of the assignment, the Supervisor will be responsible for the following:

- Development of Supervisor's Quality Assurance System;
- Development of project management information system;
- Development of detailed organization chart;
- Definition of methodology, timing and deliverables for the various activities;
- Creation of regular meeting schedule and setting of acceptance procedure;

- Monitoring and checking of design implementation.

It is assumed that all official communications will be in writing. Verbal instructions could also be possible but the same will be confirmed in writing immediately. Minutes of all meetings drafted by the Project Manager will be kept and will be signed by the principal attendees as a true record of the proceedings of the meeting. These meeting records will provide a valuable reference if any controversy arises in the future.

Original outgoing and incoming correspondence together with attachments will be registered and filed into the main IN and OUT files and categorized as necessary (e.g. "for verification and comments"; "for information only"; "Verified, no objection"; "Verified, comply with comments"; "Verified, rejected: re-submittal required", etc..).
IN coming and OUT going correspondence will be stamped to identify the person who has drafted the letter and those team member that will receive it for information.

A.2.9.1 Testing

The Supervisor shall verify beforehand the program and procedures for tests to be carried out throughout the construction period. General testing principles, their frequency, typology, methodology, reference standards shall be reported in the Technical Specifications of the Project and detailed in relevant Method Statements and in the Inspection & Test Plan. Technical Specifications shall include the following principles:

- The Contractor will be responsible of all tests for the Quality Control to be performed in connection with the materials used for the construction of the Works.
- The Contractor shall supply, install, operate, maintain and remove at the end of the Works a site laboratory that will have to function as Quality Control of the Works.
- The laboratory shall be run by Contractor's personnel experienced in sampling and testing of materials, and quality control.
- The Contractor's laboratory shall be designed for performing most of the sampling and testing required in the Technical Specifications. Tests on samples and materials, which cannot be done at site due to impracticality of retaining such specialized equipment, shall be performed by reputable, competent and certified Quality Assurance laboratories.
- Quality Assurance of the Works shall be carried out by the Contractor who will manage the site laboratory.
- The Contractor shall be responsible for the handling and transport of all materials required by the Quality Assurance Laboratory.

Main tests to be verified by the Supervisor, before and during the execution of the works shall include, but are not limited-to, the following:

- foundation materials;

- RCC and its components;
- embankment materials,
- conventional concrete;
- shotcrete;
- grouting materials;
- steel bars, steel mesh, dowel, bolts, tendons, pipes;
- hydro steel structures,
- hydro mechanical/electrical equipment,
- materials and/or assembled parts, etc.

Some comments on the above tests are given in the Quality Control Section reported in the following paragraphs.

A.2.9.2 Quality Control of civil works

GENERAL

The civil works object of construction monitoring include:

- diversion works;
- RCC dam;
- spillway;
- Middle Level outlets;
- Intakes;
- Power tunnels;
- Gate Shafts;
- Surge shaft;
- Powerhouses;
- Outlet works;
- Switchyard;
- Etc.

The Supervisor's experts will provide the following services:

- check progress of supply, works construction and all operations against contract program requirements, and request remedial measures by the Contractor in case of delays or other deficiencies;
- supervise and check all tests and other quality control measures required by the Inspection & Test Plan submitted by the Contractor, up to the issue of the final acceptance certificate;
- check the equipment and systems for the permanent works, including dimensional checks on fabrication details for installation and erection operations;

- issue special instructions to the Contractor in case of emergency and, if necessary, plan consequent measures with the Project Management;
- supervise proper performance of the Contract, including assistance in finding an amicable settlement of disputes and claims with the Contractor, avoiding resort to litigation and/or arbitration;
- give advice as dictated by the course of the works, including requests for modifications;
- accounting and administrative control: checking monthly measurements with the Contractor, verifying Contractor's monthly progress statements and accounts, and issue of monthly certificates of payment;
- issue of all provisional and final acceptance certificates for the works;
- prepare monthly and other reports.

The test types and the required frequency of testing will be provided in the Contract documents, provided that the Supervisor may review the testing programme based on the analysis of the results obtained.
The Supervisor will verify all results.
In all cases, samples will be representative of the "real" materials to be used in the Works and sampling will be carried out in accordance with the requirements of reference standards, or an approved equivalent.
The whole process of monitoring will be in accordance with the established and approved Quality Assurance Plan; non-conformities will be identified by the Supervisor and corrective action procedures initiated and subsequently monitored.

The process of day-to-day inspection and monitoring of the Works will generate routine instruction from the Supervisor to the Contractor as the essential tool of the Supervisor to exercise control for ensuring that quality and operational requirements are being maintained in accordance with contract conditions.

All instructions will be given in writing. Instructions pertaining to variations of the Works will also be issued in writing. Only in cases of emergency, the Contractor will be required to act on verbal instructions, which will be followed by written confirmations as soon as practicable.
The site staff will make the appropriate inspections and tests as the works progress and as may be offered by the Contractor for acceptance in accordance with the procedures established under the Quality Plan for the project. Independent audit checks, through visual inspections and material testing, will also be undertaken by the Supervisor to verify that the QA records provided by the Contractor are in accordance with the statements being made by the Contractor.

In the following pages there are provided some notes on the major issues and Quality Control relevant to some of the most critical Works of Batoka Project that is the RCC dam. Comments on Underground Works are given in the paragraph relevant to Geological Inspection.

RCC DAM

Dam Project includes an RCC main dam with a gated spillway on the top of the dam and Middle Level outlets embedded in the dam body.

RCC is a technology that involves:

- Materials (cement, aggregates, admixtures, pozzolan, etc.);
- Equipment/plants (crushing plants, cooling system, batching plants, conveying system, etc.);
- Procedures (delivery, spreading, compaction, curing, joint treatments, cleaning, etc.)

and may present several interferences with other work phases like:

- Foundation mapping and treatment;
- Cutting of contraction joint;
- Positioning of waterstops;
- Lifting of formworks;
- Installation of instrumentation systems and embedded parts
- Etc.

To achieve the quality of the works and the planned production rates it is mandatory to guarantee a close coordination between all the parties involved in the activities mentioned above and an extensive knowledge of the various processes.

As detailed further on, the Quality Control Program of the Contractor shall include also:

- a Quality Control for the Staff Training and
- Training Courses for the RCC Staff and Workers involved in the execution of the works.

These training activities shall start before the starting of RCC placing activities in order to anticipate as far as possible the achievement of learning-curve peak.

This point is extremely important and sometimes under-estimated since, usually, the most critical phases in building an RCC dam are those occurring at the beginning of the works. Higher hydraulic loads and higher stresses in fact characterize the RCC placed at the bottom of the dam. In this zone, also the interfaces with foundation and its treatment are generally the most challenging.

The Contractor shall make available on site a suitable laboratory equipped for the manufacturing, curing and testing of a large number of RCC cylinders and define a detailed Mix design test programme to be carried out well before the start of placement of RCC and the starting of Full Scale Trials.
The aim of this test programme is to identify the most suitable mixes to be used in the different zones of the dam and to confirm the design parameters for the dam including but not limited to: uniaxial compressive strength, tensile strength, density, deformability modulus, VeBe time, initial setting time, thermal properties, etc.

Most complex tests, especially those related to the thermal properties of the mix, may be carried out in international renowned laboratories.

Technical Specifications and relevant Method Statement shall include the details and the sequence of all the main items that concur in the manufacturing of RCC, including:

- Cement (type (generally, Frequency of chemical, physical, hydration heat, tests from factory and on-site, transport and stockpiling facilities);
- Aggregates (Source, type and frequency of tests, acceptance criteria, aggregate production plants, shape, gradation, transport and stockpiling);
- Water, filler, admixtures;
- Layout of RCC plant, peak and normal capacity; methods and equipment for handling, mixing, transporting, spreading, compacting, curing, protecting and cleaning

Moreover, it shall be defined:

- Methods for temperature control;
- Criteria for Joint classification (e.g. Lift Quality Index) and relevant treatments;
- Sampling facilities;
- Programme of Full Scale Trials;
- Types and frequency of tests, both at batching plant and on site.

The Supervisor shall joint the Contractor during the execution of all these tests and shall possibly perform is own and independent evaluation of test results, joints classification and mapping.

The Contractor shall establish and maintain an effective quality control program for RCC to ensure compliance with the Contract requirements and for maintaining records of control, including tests and inspections, their findings, and remedial actions taken when necessary.
The Contractor's program will be established and run under the supervision of a full time experienced Quality Control Engineer who will review and approve all activities concerning the production of materials, planning and scheduling of construction activities for placing RCC and the running and evaluation of RCC tests.
The RCC Quality Control Program shall include but not be limited to the following: aggregate manufacture and gradation, moisture, batching requirements and mix proportions at the batch plant, mix delivery, compaction, joints (insuring adequate materials are on hand), embedded items, erection of precast facings, and all other tests and inspections required by Specifications.
Quality Control Program shall include also Quality Control Staff Training as well as RCC Staff and Workers Training Courses for the execution of the works.

Tests at prototype scale are essential to verify the "real" characteristics of RCC. In fact, the entire production process, from batching plant to dam site, clearly affects the final quality of RCC in such a way that laboratory tests or even Full Scale trials may be not completely representative or the real performances of the dam.
In this sense following kind of tests are recommended to be carried out on site to better characterize the RCC dam:

- Large scale shear block tests on samples taken directly from the dam body using large diameter diamond circular saw;
- Detailed log of RCC cores drilled periodically in the dam body and statistical analysis over the distribution of segregated area, bonded on not-bonded lift-joints, etc.;
- Laboratory tests on RCC cores drilled in the dam body, including UCS, direct tensile strength, lift-joint shear strength, permeability;
- Mapping of U/S face of the dam soon after formwork removal in order to detect any defect;
- Mapping of dam permeability at large scale, executed by means of water tests on dam drain holes.

The above kind of prototype-tests can be required to be included in the RCC Quality Plan of Contractor.

A.2.9.3 Quality Control of Hydro-Mechanical and Electrical equipment

As indicated in the previous sections, the Contractor will submit, for the Supervisor's approval, the Detailed Design including all detailed data, calculations, explanatory documents and drawings necessary for manufacturing and testing of the Hydro-Mechanical and Electrical equipment.
These documents, duly reviewed by the Supervisor, should form the basis of the contractual procurement of the equipment from the manufacturers.

It is expected that the information to be supplied by the Contractor, and verified by the Supervisor during this phase, will include the following items:

- Hydro-mechanical Equipment:
    - up-to-date layout;
    - hydraulic design calculations for the full range of operations;
    - stress analysis of components;
    - tables with characteristics of main parts (flow, dimensions, weight, etc.);
    - critical operating conditions;
    - sequences, time, methods for assembling parts and erection rigs characteristics;
    - materials to be used for the various elements of the supply stating their durability;
    - maintenance ways and ease of performance;
    - quality control tests and operation safety.

- Hydro-electric Equipment:
    - up-to-date layout;
    - design calculations for stress bearing structures;
    - tables with final rating of main parts;
    - sequences, time, methods for assembling parts and erection rigs characteristics;
    - turbine critical speed;
    - requirements for unit bearings;
    - characteristics of cooling systems;
    - transport fittings and protections;
    - materials to be used for the various parts of the supply stating their durability;
    - quality control tests;
    - maintenance ways and ease of performance;
    - operation safety.

- Electrical Equipment:
    - generator design;
    - materials and technology;
    - quality control tests;
    - equipment ratings and test certificates for major plant components;
    - fault level calculations;
    - voltage regulation analyses;
    - reactive power control;
    - motor characteristics, duty cycle, starting performance of major and essential;
    - auxiliaries, selection of power cable ratings;
    - insulation, segregation and separation of power and control cables;
    - fire prevention measures;
    - design, types, performances and characteristics of HV equipment and electrical devices;
    - single line diagrams;
    - design and calculation of connection to HV line and substation.

- Electrical Instrumentation and Control:
    - compatibility among different suppliers' equipment and layouts;
    - P&I diagrams;
    - interface with ICS/LDC remote dispatching systems, if any.

The Supervisor will follow the different phases of the detailed design and will oversee the activities of the Contractor, as well as investigations and inquiries realized at site.

The Supervisor review will consist in examining the various documents to verify the:

- correct and adequate selection of materials depending on their specific destination and according to the Client's Requirements and international practices;
- proper dimensioning of the various parts with calculated maximum stresses within internationally accepted limits for the various load conditions and cases;
- appropriate construction design of main equipment adopting latest high-quality practices and especially facilitating assembly and future maintenance;
- satisfactory choice of performances and ratings of different components, with sufficient safety margins and adequate overcapacities to improve reliability;
- proper interface of the equipment with the 1st phase concrete.

MANUFACTURING

During manufacturing, the Supervisor will check the quality sensitive activities in the Contractor's Head Office, in the Manufacturers' workshops and at the construction site, reporting any significant discrepancies and suggesting corrective follow-up actions.

It will be required to verify that the inspection procedures submitted comply with Contract requirements and with specified quality standards of internationally accepted codes.

In particular, the controls will encompass:

- planning procedures;
- document control;
- verification and control of materials, parts and components, welding capability of specialists;
- control of welding and/or special processes;
- inspection for test control;
- control of measuring and testing equipment;
- handling, storage and transport records.

In order to achieve the above, the Supervisor will undertake the following actions:

- review the Quality Management System of the Manufacturers;
- inspect, where necessary, the major equipment and components during manufacturing at the Manufacturer's factories;
- evaluate the test results for materials and respective certificates issued by the Manufacturer's suppliers. Should critical situations be identified, the Consultant will prepare a specific report on the matter in order to allow the Owner to take action;
- coordinate, expedite, schedule and report results of workshop inspection visits.

Sufficient records will be developed and maintained so that the results of the inspection are easily identifiable and traceable.

Before starting the construction works, the Supervisor will conduct inspections, audits and factory tests witnessing of materials and equipment during the manufacturing and procurement phase, with particular

attention to type tests and acceptance of the main components, as well as "routine" and "sample" test relevant to the prototypes, ensuring the satisfaction of the features requested by technical specifications.

The Supervisor will participate in factory testing of major equipment, and preparation of corresponding acceptance certificates. The main equipment subject to this monitoring activity will be determined by agreement with the Supervisor.
The Contractor will submit the detailed program of work developed in all its phases (project, production, transportation to site, installation and testing) for verification and validation. Finally, the Supervisor will make sure that necessary operation and maintenance manuals, documentation, training, etc. are supplied by the Contractors to remove bottlenecks and expedite shipment
The Supervisor will review the results of the turbine model test report, to assess that the guaranteed equipment characteristics and performances have been satisfied in respect to the EPC Contract, as well as to confirm that the operating characteristics of the proposed machines are within acceptable levels.

SITE INSTALLATION AND ERECTION

The Supervisor will supervise the Contractors' activities for the installation of the electrical, electromechanical and hydro mechanical equipment since their arrival on site up to their commissioning. Basis of this activity will be a work breakdown structure whereby the entire project is broken down into successive small phases.

A schedule will be built up for each project section (e.g. power house equipment, substations, etc.). At the outset, the schedule will be set up based on the submitted contractor's schedule.
Whenever dam and power houses fall within different contracts and contractors, the schedule of each work will be coordinated with the others, and possible interferences and interconnected activities prescribed to be managed accordingly.

The schedule will identify the critical path and will be regularly updated as construction proceeds. Should maintenance of the critical path be endangered, the schedule will facilitate early recognition of this so that corrective action can be implemented in good time.
Once the schedule has been established and construction is in progress, our on-going work relating to this activity comprises monitoring and controlling. During the execution of the works, the Supervisor will closely check all operations so that those items not complying with the requirements are identified and consequent corrective actions suggested; where necessary, action will be taken to prevent recurrence of nonconformity conditions. During the implementation of civil works and installation of electrical, electromechanical and hydro-mechanical equipment, the Supervisor will closely monitor the progress of each phase of the Project.

The Supervisor will monitor the progress of the activities to have a complete, overall picture of the situation and agree on the activities to be performed.

During the supervision of erection, the Supervisor will check and validate the course of the implementation of the work, the compliance with the project and with the contracts and the quality assurance of:

- temporary structures built in situ;
- work schedule;
- possible additional work;
- any recommended changes required by the Supervisor;

and will perform the following activities:

- interpretation of drawings and specifications for the conformity of assembly work to contract;
- checking of the test procedures for facilities and equipment;
- consideration of any changes in the field that may be required;
- monitoring progress in respect to milestone dates and events;
- coordination and inspection of the detailed installation activities to ensure continuous and effective progress of all work and ensure compliance with specifications;
- monitoring and verification of all testing and other quality control measures required by the contract until the issuance of the Performance Certificate.
- Monitoring of the Contractor activities in respect to the compliance with local and international site safety and environment standards and regulations.

A.2.9.4 Geological Inspection

The following general principles shall be followed during the implementation of the works related to excavation activities:

- mapping methodology, procedures and their representation in a graphical format will be primarily checked, reviewed and agreed with the Supervisor, who shall have a geological team available to make visual inspections of all excavations jointly with the Contractor as well as independently;
- the Supervisor shall ensure that the geological mapping will be carried out both for open air excavation and underground excavation beforehand the excavation itself is covered by embankments, concrete, temporary and permanent support works etc.;
- the Supervisor has to ensure that no foundation surface shall be covered by any material before the mapping mentioned in previous point has been approved and the prescribed treatments of excavated surfaces completed;
- The Supervisor's geological team, together with the Supervisor's Resident Engineer, shall give all recommendations to the Contractor(s) for the re-design of foundations due to un-foreseen rock conditions in the course of excavation, if needed.

UNDERGROUND WORKS

As far as power waterways tunnels excavation is concerned, the Contractor will carry out a comprehensive investigation campaign in order to obtain sound geotechnical parameters and a safe design, which will include:

- excavation methodology;
- excavation phasing;
- monitoring system (e.g. convergence);
- temporary support works;
- permanent support works.

The Supervisor will:

- review the investigation program and test results;
- review Contractor's design;
- perform periodical geo-structural inspections during the tunnel excavation phases in order to verify the geotechnical classification of the various stretches and the appropriateness of the temporary and permanent supports designed by the Contractor.
- perform periodical inspections to verify that the as-built corresponds to the design;
- perform rock-bolts pull out tests
- check tunnels sizes and verify their dimensional accordance with the project;
- Perform a topographical survey of the tunnels alignments;
- Analyze data of the monitoring system.

OPEN AIR EXCAVATIONS

Surface surveys are required to control the status of works and the geological conditions of the foundations. As explained in previous paragraphs, the possibility to make use of drones for the survey shall be explored, and welcomed, since it allows to obtain georeferenced photos of the inspected area and a Digital Terrain model, allowing to obtain a complete 3D model of the surveyed area elaborating all the sequential photos with engineering software.

Combining the site investigations (visual inspection, Schmidt hammer, geophysical tomography at small scale, etc.) with the superficial survey, it is possible to evaluate the main mechanical characteristics of the rock, such as:

- GSI (Geological Strength Index);
- RMR (Rock Mass Rating)
- Uniaxial Compressive strength
- rock type;
- degree of weathering of the rock;
- discontinuities in rock mass;
- etc.

and record them in a geo-referenced 3D database.

Drone and geological survey are generally carried out in local areas of limited extension, following the progress of the excavation works.
Since the information acquired with the proposed methodology is geo-referenced, it is always possible to assemble the local mapping in larger scale drawings obtaining a useful view of the foundation geology at the scale of the project.

A.2.9.5 Monitoring, Instrumentation System and Emergency Preparedness Plan

About this topics reference shall be made also to other dedicated sections of this Dam Safety Plan.

The detailed design of Monitoring and Instrumentation systems shall be carried out by the Contractor (requirements of the Instrumentation system are to be detailed in the Final Design phase) and verified by the Supervisor.
Supply and Installation of Instrumentation system shall conform to the following principles:

- instrumentation shall be of high quality, realized according to the best practice and international standards.
- all the instruments shall be subject to Contractor's acceptance procedure of inspection and testing at their arrival at the Site.
- acceptance of the instrumentation devices after installation shall derive from special procedures to be defined by the manufacturer/supplier and agreed with the Supervisor.
- testing, calibrating, installing and commissioning of the monitoring system shall be provided by supplier's skilled and qualified personnel, under the Contractor's coordination.
- all instruments shall be accompanied by Technical Specifications, Procedures for the installation and use; calculations and interpretation of the measured data (calibration curve, calculation formula and form, example of calculation); acceptance testing procedure and maintenance manuals (including list of recommended spare).

Design and review of the instrumentation system shall be ready before the commencement of the works in order to anticipate the identification of any possible interference with civil works and to plan all the installation facilities for correct placement and wiring connections of the instruments.
Moreover, some instruments shall be installed from the early stage of works in order to acquire all the necessary data regarding the performance of the Project.

The Supervisor shall review the Monitoring and Instrumentation system considering the guidelines suggested by international codes and standards such as:

- "Instrumentation for Concrete Structures" US Army Corps of Engineer, 1987;
- "Embankment Dam Instrumentation manual" USBR, 1987

- "Instrumentation of Embankment dam and levees", US Army Corps of Engineer, 1995;
- "Automated dam monitoring system", Icold bulletin 118, 2000
- "Dam foundation: Investigation, treatment and monitoring" Icold bulletin 129, 2005

Of course, the Supervisor will make use of his expertise to tailor the Monitoring System to the specific requirement of Batoka project.
The Supervisor shall be involved at the initial readings of each instrument to ensure that the methods and procedures used are correct and allow the determination of reliable ''zero'' values.

The Supervisor shall verify that the readings are carried out at the prescribed times and intervals, the procedures and methods of reading are correctly applied, and results are correctly registered, filed and analysed. During the First Impounding phase, as detailed in the First Impounding Programme, the rate of readings shall be increase and the data shall be elaborated in real time in order to verify the thresholds values corresponding to Alert and Alarm conditions.

Finally, the Dam Break analysis and the Emergency Preparedness Plan prepared by the Contractor.

A.2.9.6 Monitoring of Construction schedule

All what follows can be refined and adjusted if and when one or more contracts will be set for the construction of Batoka Project. In the case Dam and Power Plants are allocated to different Contractors, relevant construction programmes shall be analysed separately as well as together as far as possible interferences and dependences are concerned.
What follows is referred in general to the monitoring activity of the construction schedule(s).

The Supervisor shall check that the Contractor maintains a proper organization of the Works in all phases in order to keep the targets of the General Work Schedule and the final commissioning of the plant.
The Supervisor shall alert the Contractor in due time, whenever the Supervisor foresees any risk of delay and shall request the Contractor to re-analyze the programs and sequences of Works. The Supervisor will suggest partial or general adaptations of the program whenever deemed necessary.
Particular attention shall be given to the co-ordinations of the activities that imply interfaces between civil works, hydro-mechanical, electro-mechanical and mechanical equipment.

In order to ensure all the activities commented upon here above are correctly, timely and efficiently carried out by all parties, procedures and schedules for meetings on site shall be established in due time. The Supervisor shall be responsible for the enforcement of these procedures and schedules once they have been mutually agreed upon.

The basis of Monitoring activity is a work breakdown structure whereby the entire project is broken down into successive small working lots, typically over five levels as follows:

- Level 1 – Project Section;
- Level 2 – Task;
- Level 3 – Sub-Task;
- Level 4 – Work Package; and
- Level 5 – Individual Work Items.

A schedule will be built up for each project section (e.g. Diversion Works, Dam, Intakes, Tunnels, Surge Shafts, Spillway, Bottom Outlets, Power Houses, Switchyards, etc.), starting from Level 5 work items and the successive upwards aggregation to the higher levels. At the outset, the schedule will be set up based on the Contractor's submitted schedule.
The schedule will identify the critical path and will be regularly updated as construction proceeds. Should maintenance of the critical path be endangered, the schedule will facilitate early recognition of this so that corrective action can be implemented in good time.

Once the schedule has been established and construction is in progress, the Consultant shall:

- monitor the rate of progress for any given work item and the amount of resources required to achieve this rate;
- control and evaluate the data received from the monitoring process to identifies the necessity of any action.

Monitoring and control thus give input to planning and result in a cycle (planning-monitoring- control-planning-etc.) which is continuously repeated throughout the duration of construction.

The Supervisor will periodically monitor the progress of the activities so to have a complete, overall picture of the situation. A valid instrument to carry out the monitoring of the progress of works will be the monthly reports and any other necessary report, as detailed below:

- Inception Report;
- Monthly Progress Reports (suggested to be integrated with short-duration drone videos);
- Quarterly Progress Reports;
- Annual Reports;
- Reservoir Impounding reports;
- Final report before waterway system filling;
- Waterway system filling report;
- Commissioning acceptance test reports;
- Specific reports on special cases as required;
- Claim assessment reports;

- Supervisor's site and home office periodic activity reports;
- Project Final completion report;
- Defects Notification report.

A.2.9.7 Commissioning programme

The Contractor shall prepare the detailed programme of site tests with the overall goal of clearing the way to a successful introduction of the plant into the existing interconnected system. The programme shall outline the approach and methodology of the start-up operations, and will be subdivided in subsequent phases:

- Pre-commissioning tests;
- Commissioning tests;
- Trial operations and
- Performance and efficiency tests.

Prior to the execution of this programme, the Contractor should submit also a preliminary version of all as-built drawings and of Operation and Maintenance (O&M) Manuals.

Throughout the commissioning stage of the Project, the Supervisor will provide a Commissioning Manager and specialists in the applicable engineering disciplines to support the activities related to commissioning, performance tests and transfer to the Owner structure of systems and equipment.

The programme should define the areas of responsibility and detail technical, administrative and safety procedures. It should also define the principles for the technical requirements associated with the commissioning of individual components or of systems, including the use of commissioning check-lists to record the checks made and the tests undertaken, the results obtained, the signatures of the EPC Contractor commissioning, Specialists and of witnesses from the Supervisor and his commissioning staff.

The Commissioning Programme to be submitted by the Contractor should also include the procedures for performance and guarantee acceptance tests, specifying acceptance criteria, test parameters, instruments to be used and instrument accuracy requirements.

The Supervisor staff, will review the Commissioning Programme and the attached procedures, checking that design requirements are complied with, methods proposed are suitable to keep operations under control and all necessary verifications are carried out.
The Supervisor shall verify that all relevant parts and components are ready for the various tests and check periodically that the time-schedule to achieve critical steps of the programme are maintained, prescribing all the remedial actions required to minimize the risk of delay.

A.2.9.8 Reservoir Impounding

The First Impounding Plan (FIP) coordinates the work activities following the closure of the river diversion and, consequently, it deals with the reservoir filling, the work scheduling, the instrumental monitoring of the dam behaviour and the procedures to be adopted according to the most likely foreseen scenarios occurring during the reservoir impoundment.
Before the implementation if the First Impounding Plan the Supervisor shall receive and approve the Reservoir Operation and Maintenance Manuals prepared by the Contractor, verifying that the procedures described in the FIP are coherent with those indicated on the manuals.

The First Impounding Plan report will be structured in a similar manner to the example given below:

- KEY ROLES
  This chapter will describe the individual roles and responsibilities of the figures who will be involved in the first impounding procedure. In particular, there will be Dam Safety Committee (DSC) that will implement the Dam Safety Program (DSP). The DSC will be responsible for all the activities related to the safety and safe performance of civil works and equipment of the project and will involve delegates of Owner/Supervisor and Contractor any other authority indicated by the Owner or Stakeholders involved.

- DEFINITION OF THE CONDITION READY TO START
  This chapter will describe all the recommended pre-filling conditions.
  In particular:
  - a reliable Monitoring System shall be set up and tested before the start of impounding in order to evaluate Dam behavior under impoundment.
  - All the equipment and manpower necessary to proceed with the plugging of Diversion works shall be available on site.
  - All the devices necessary to release ecological flow, control the rising of the reservoir levels, protect the sensitive areas or portions of the works shall be ready and fully operative.

  The readiness for the start of impounding activities will be verified by a joint inspection of the DSC based on the recommendations given by the Supervisor.

- DESCRIPTION OF THE IMPOUNDING
  When all the activities indicated in the previous chapter have been correctly carried out, the DSC orders the start of the impoundment.
  This phase of impoundment will proceed to the full filling of the reservoir or pre-defined target levels unless differently prescribed by DSC based on the observed behavior of the dam.
  During the impounding all required measurements will be made (deformation, settlement, seepage, etc.) by the Contractor at the presence of the Supervisor and transmitted to the DSC.

- CONTROL PLAN

  This chapter will describe the recommendations for the acquisition of measurements, which allow assessing the behavior of the dam and of the main works and to verify the hypothesis at the base of geotechnical/hydrogeological/structural models assumed at the design stage, particularly:
  - level of the reservoir;
  - seepage (dam drainage);
  - seepage turbidity;
  - piezometers readings;
  - embankment settlings;
  - inclinometers readings;
  - topographical survey of the dam structure;
  - etc.

- INSPECTION PLAN

  This chapter will describe the inspections that will have to be carried out with regard to the works and downstream of Batoka Plant

- ROUTINE and ALERT CONDITIONS

  This chapter will describe the normal and threshold values of the monitoring instrumentation. The following procedures will also be defined:
  - Routine Procedure;
  - Alert Procedure;
  - Controlled Drawdown Procedure;
  - Emergency Drawdown procedure.
- CONTINGENCY PLAN

  Finally, this chapter will describe the plan of action to be taken in case of an impending catastrophic failure of the dam or a large and sudden release of stored water.

The Supervisor shall verify and approve the sequence of operation described in the First Impounding Plan to proceed with First Impounding, including monitoring program, definition of routine and alert conditions etc. and shall actively participate to the impounding process through the Dam Safety Committee.

Some of the aspects above described are already developed, at feasibility design stage, in other sections of this Dam Safety Plan.

A.2.9.9 Testing and Final Control of Equipment

The Supervisor, under the guidance of the Commissioning Manager, will take note of any faults or defects detected during the commissioning tests performed by the Contractor(s) and will monitor the interfaces with the plant operation.
Commissioning and testing procedures will include acceptance criteria, test parameters, instruments to be used and instrument accuracy requirements.

The commissioning tests will include performance, functional and operational tests. Tests on completion should usually be carried out in the following sequence:

- pre-commissioning tests, including appropriate inspections and dry or cold functional tests to verify the physical reliability of the individual equipment elements and their ability to function within their respective systems as per design;
- commissioning tests, including specified operational tests to demonstrate that the works or sections of them can be operated safely and as specified under all normal and exceptional operating conditions;
- trial operation tests (Trial Run), to demonstrate that the works or sections perform reliably and according to the contractual conditions and requirements.

At the completion of the erection and installation works, the Contractor will start carrying out the "dry" (or "cold") and "wet" functional tests, in accordance to the program submitted for the Supervisor approval.

The Supervisor is called on the field to carry out, jointly with the Contractor, the so-called "Final Inspection" of the installed equipment and plants. The aim of this inspection is to verify the compliance of the erection works to the project requirements, and to identify those items, which need small adjustment, settings or fine-tuning in view of the first energization and Trial Run of the plant.

Acceptance certificates will be issued by the Supervisor at the end of the successful acceptance tests of each unit and at the commissioning of any facility.
Acceptance Committee, consisting of delegates members from the Supervisor and the Contractor will be set up to fulfil these tasks. The Supervisor is responsible for all the preparatory verifications prior to the commissioning. Ministry of Energies of the power plant country who issues these certificates of acceptance, have typically the mandate to oversee that the facilities:

- conform with the approved final design and with the regulations and standards applicable internationally,
- can be integrated to the national grid
- are secured with respect to safety.

A.2.10 SERVICES DURING DEFECT LIABILITY PERIOD

What follows is a general guideline that shall be adapted to the type of construction contracts that will be put in place.

During the Defects Liability Period (DLP), when the Contractor is typically bound by the Contract to remedy any kind of defective works by his fault, the Supervisor will make available his own experts for hydro-mechanical, electrical equipment and Civil Works.

Supervisor' specialists will inspect the Works to determine precisely the actions that the Contractor took in accordance to the Contract and will inform the Contractor in writing about this.

During the Defects Liability Period the Supervisor will acting closely to the works for:

- the finalization of the remaining works at the date stipulated by Contract (List of Outstanding Works at the Taking Over);
- rectification of all defects during the warranty period with issuing of clearances for defects rectified as well as a prolongation of warranty for major parts, if applicable. The punch list is updated on the basis of the progress of the Contractor's activity
- preparation of a project completion report, which acceptance imply the end of the Supervisor's assignment versus the Contractor;
- overseeing compliance to contractual obligations;
- fixing of all the defects and the damages caused during the Defects Notification Period.

The Supervisor will carry out inspections as necessary, after commencement of DLP, and will prepare brief reports on any issues or defects discovered / repaired by the Contractor. Should be necessary, the Supervisor could require repetition of any test and check to be carried out by the Contractor. The Supervisor is also called to witness tests and accept relevant documentation.
The Contractor will update, accordingly to any repair work executed, the as-built drawings, documentations and O&M manuals.

The list of the remaining works to be executed during the Defects Notification Period is mentioned at the Taking-Over Report; these works should be completed within the respective deadlines. The Supervisor will monitor and notify the Contractor about any defects occurred during the Defects Notification period.
The Supervisor will inspect and verify the completion of the remaining as well as the remedying works to be executed. He will monitor the maintenance works recommended in the Operation and Maintenance Reports for the Defects Notification Period, elaborated in accordance to the Supervisor recommendations.

The Supervisor, following the approved procedures, will assist in the issue of the Final Acceptance Certificate at the end of Defects Notification Period (DNP) with respect to:

- Completion of Outstanding works as listed in the report which accompanies the Taking Over certificate;
- Listing of Construction Works with defects;
- Time schedule and activities of remedial actions
- Failure of the contractor to remedy defects;
- Conditions for approval of the site demobilization;
- Current stage of existing demands and claims;
- Previous conditions for the issuance of the Final Acceptance Certificate (FAC)

The Supervisor will ensure that the conditions for the issuance of the Final Acceptance Certificate (FAC) are accomplished at the end of the Defects Notification Period.

A.2.11 HANDLING OF CLAIMS

What follows is a general guideline that shall be adapted to the type of construction contracts that will be put in place.

The Supervisor will provide all efforts to settle any conflict with the Contractors. In case any dispute, controversy or claim arise with the Contractors relating to or arising from the relevant contractor's agreement (contract), or the breach, termination or validity thereof, the Supervisor will be aimed to resolve such matters amicably through negotiations.

Should such negotiations not lead to a mutually acceptable resolution of the dispute, the dispute, controversy or claim shall be settled according to the general conditions of contract and/or special conditions of contract. The Supervisor will verify that notice of claim has been given in due time after becoming aware of the situation giving rise to the claim. Should the notice be given too late or be not supported by the required information, the Contractor will not be entitled to any extension of time or additional payment.

After the inspection, the Supervisor will prepare an assessment of the Contractor's contemporary records and with his preliminary conclusions about the potential outcome of the claim and will define any instruction to issue to the Contractor about further contemporary records.

A.2.12 MANAGEMENTS OF PAYMENTS

What follows is a general guideline that shall be adapted to the type of construction contracts that will be put in place.

The Contractor is generally required (for instance as per typical FIDIC Conditions of Contract) to submit periodic applications for Interim Payment Certificates, together with supporting document and calculations, of the value of Works executed for which he considers himself entitled to payment. Each application should be based on the joint measurement and agreement of quantities of work performed by the Contractor and accepted by the Supervisor.

The Contractor will be required to submit progress payment requests, based on measurement, on a scheduled basis (usually monthly) throughout the construction phase. The supervision team will maintain adequate records to review and check the measurement and progress payment requests in detail. The supervision team will also review payment requests and prepare payment certificates recommending that a certain progress payment amount is due.

FIDIC places responsibility upon the Client and his Client's Representative (if any present) to perform a measurement after reasonable notice from the Contractor that he requires any part of the Works to be measured for purpose of interim payment.
FIDIC requires the Contractor to attend and assist the Client's Representative in making such measurement, failing which the Contractor has limited opportunity to dispute the Client's determination. In practicality and to avoid disputes the Supervisor will establish procedures whereby the intent will be jointly to perform and agree the measurement prior to the Contractor's submission of each and every monthly statement.

Based upon the information he has gathered during the month in question, and in accordance with the Conditions of Contract, the Client's Representative will determine monthly interim conformity to payment certificates for work completed and approved. Parallel with these activities, the Contractor is required to submit to the Client's Representative monthly statements, supported by relevant documents, calculations, sketches and drawings, of the works executed for which he considers himself entitled to payment.

In his preparation of Interim Payment Certificates, the Supervisor will apply the Contract provisions in respect of:

- schedule and percentages on the Bill of Principal Quantities of the Permanent Works established for civil, hydro mechanical and electrical Works stipulated in the EPC contract
- provisional and Lump Sum items;
- value of any day-works performed for which the Supervisor has issued prior instructions;

- payment for work undertaken by nominated sub-contractor(s) (such payment requires evidence that prior payment has been made to the nominated sub-contractor(s));
- materials on site;
- deductions in respect of recovery of any Advance Payment made to the Contractor;
- deductions in respect of retention monies;
- application of any adjustment factors allowed for in the Contract to allow for escalation of prices;
- deduction in respect of any statutory taxes or levies applicable under the Contract;
- deduction in respect of any Liquidated Damages applicable;
- any corrections required to previous Interim Payment Certificates;
- any interest to which the Contractor is entitled as a result of previous late payments; and any other costs to which it has been determined that the Contractor is entitled; for example claims and/or extensions of time.

The Supervisor will ensure that Interim Payment Certificates will be fully supported by back-up calculations (and sketches where necessary) indicating the location and scope of works measured and certified for payment.

After receiving the Performance Certificate, the Contractor shall submit to the Supervisor a draft of the final statement with supporting documents in an approved form indicating the value of the works done in accordance with the Contract and any further sums that the Contractor considers to be due to him under the Contract.
After the review of the Supervisor, the Contractor could be requested to provide additional information and then prepare and submit the Final Statement.

A.2.13 TRANSFER OF KNOWLEDGE

The optimal management and operation of dams over the years can be pursued only by employing highly qualified engineers and technicians and with a broad experience.
In the case the Owner will recur to the assistance of an international consultancy firm for Batoka Supervision, the training program and transfer of knowledge and technology are important aspects of such assignment.
The main goal of this task is to use the opportunity of the Design Review and Construction Management Supervision of Batoka Plant to transmit the international experience of the hired experts to Client Project Team staff nominated by the Owner.
What above and what follows is indicated for the Dam Owner, but as far as necessary in the case different contracts are put in place for the two Plants of Batoka Project, the guidelines provided can be applied for specific training activities for each of the Plant (and relevant Owners or operators).

In that case what is most important is that the Consultant will provide an on-the-job training to the Client's personnel on all aspects of the Consultancy during construction and commissioning stages, as illustrated here below.

Furthermore, the training program will be composed of workshops and presentations where not only Owner's Project Engineers and technical staff but also external personnel, for example, university students may actively participate.

A.2.13.1 Approach and Methodology

At the beginning of the assignment, the international Consultant will establish an updated training program for Owner's approval. The timing and content of the training program can be modified at Owner's request in order to benefit in the best possible way the transfer of knowledge at different stages of the project implementation.
The organization of the training program will basically include three major activities:

- on-the-job training to personnel of the Owner to ensure hands-on experience during construction and commissioning of the dam;
- presentations carried out by Consultant experts during their missions on site;
- technical workshops at the different milestones of the project

A.2.13.2 On-the-Job Training to Owner's Personnel

This activity will be carried out through continuous day-to-day collaboration of Owner's staff and consultant's experts, working together at the site.
The Owner will typically provide a Core Team of Assistant Engineers plus an Additional Staff of Assistant Engineers to participate to the Consultant's project team while working at the project site.

The training program will be designed specifically to the needs of staff. The rhythm and style of learning will be adapted to the needs of participants.
The training will be supported by examples from the concrete experience of the Consultant, illustrating the critical aspects encountered in the implementation of major dam projects. The presentation of existing cases will be extremely useful for the transfer of knowledge and will be done using videos, photos, drawings, diagrams, etc.
During these on-the-job training sessions, engineers will transfer to their counterparts the technical knowledge on many aspects related to construction, assembly, testing and commissioning of hydropower plants and transmission lines.

This on-the-job training will be specifically organized in different phases of the assignment, in order to include the following key aspects especially valuable for the construction and commissioning of the dam but also administration of the works contract:

- General Dam Engineering, in particular foundation treatment, RCC dams, embankment dams;
- Geotechnical mapping of underground and open-air excavation;
- Electrical and Mechanical engineering;
- Hydraulics of spillways;
- Dam Monitoring and Maintenance;
- Site supervision, management of claims during dam construction and dam rehabilitation projects;
- Quality Assurance Plan procedures during dam operation and maintenance
- Optimization of the design of the main components of the project;
- Environmental and social issues related to construction supervision, best international standards.

Moreover, during construction controls, the Consultant can train the Owner personnel on use of instruments (such as Differential GPS, drones, sclerometres, Permeametres, Flow measurement vessels, etc.).

A.2.13.3 Workshops and Presentations Carried out by Consultant Experts

This activity will be carried out especially through the presentation of case histories experienced by the Consultant that can be similar to the Batoka Project case.

It will be defined in collaboration with the Owner the detailed program of the proposed workshops and short presentations, considering the field specializations of the Owner's personnel and Owner's needs.

A.2.13.4 Technical Presentation at the Different Milestones of the Project

For each milestone of the project the findings of all the Consultant's activities will be illustrated through a specific presentation to which will follow a debate/discussion.
Together with the main findings of the studies, the most critical aspects will be especially illustrated in detail.

PART B - INSTRUMENTATION PLAN

B.1 INTRODUCTION

B.1.1 CONTENT AND STRUCTURE OF THIS PART

This is the PART B "**INSTRUMENTATION PLAN"** of the Batoka Dam Safety Plan.

This part of the document outlines the operation and maintenance activities and procedures relevant to the Batoka Dam MONITORING SYSTEM.

It describes the monitoring system of Batoka Dam, and it contains the instruction for measurements to be collected by the instruments, for their presentation, use and assessment.

The part B is divided in two sections:

- DAM INSTRUMENTATION OPERATION AND MONITORING
  describing (at feasibility Design level) the Batoka instrumentation and the monitoring activity.

- DAM INSTRUMENTATION MONITORING RESULTS ASSESSMENT
  providing indications on how to deal with the results collected by the instruments, together with some guidelines for the instrument monitoring assessment.

The drawings showing the location and relevant bill of quantity of the instruments foreseen will be provided in the feasibility design report.

This document shall be integrated and developed, when the detailed design and construction will be carried out, annexing the following:

- Detailed design of Batoka Plant instruments
- As-Build drawings showing the location and name of each instrument
- Instruments installation sheets (Manual and Automatic)
- Instruments data sheets & manuals
- Instruments Calibration sheets and certificates
- Software manuals

This part of the report is in fact specifically focused in the instruments monitoring activities. The whole Operations and Maintenance Plan for the Batoka Plant is provided in the Parts C and D of the report, to which reference is made for all the operation and maintenance activities and procedures relevant to the Batoka Plant.

B.2 DAM INSTRUMENTATION OPERATION AND MONITORING

B.2.1 INTRODUCTION

This chapter describes the monitoring activities that shall be carried out during the operation of the Batoka Plant.
They are referred to the period of operation of the plant, they are applicable also for the dam construction and first impounding periods (as far as applicable), for which any further specific instructions can be added during the detailed design and construction process.

In the following paragraphs the instructions dedicated to the dam instrumentation monitoring for the plant operation period are provided, describing which measures shall be carried out with which instrument, where and when.
In the next chapters there are indications on how to evaluate the results gathered by the instruments, and the instructions on how to proceed in case of routine or alert conditions.

All that follows is at feasibility design stage of knowledge, therefore to be considered preliminary and to be detailed and updated before coming into operation.

B.2.2 INSTRUMENTS CONTROL PLAN SUMMARY TABLE

For an easier control and implementation of the monitoring procedure a summary table is to provided indicating all the instruments to be used on site, along with the type and frequency of measures to be carried out.

Here below a format for this table is provided, with first indications on instrumentation, to be integrated during the detailed design and construction process.

The instruments object of monitoring, that will be recalled in the table below, are assumed will be illustrated in detail in relevant drawings before the plant enters in operation, and will make part of this document and will provide, where possible, indication of the number and position of each type of instrument installed. All the as built instrumentation drawings will be documented inn dedicated documents.

All the prescriptions mentioned in the table and in the following paragraphs refer to the standard systematic monitoring activities. Prescriptions can be modified as needed if measurements show some anomalous trend or some indication of concern for the dam stability.
The routines conditions and alert conditions are detailed in the second paragraph of the next chapter.

Initials		Instruments Type	Position			Scope	Notes	Monitoring schedule (1)	
			Area	Ref. dwg	Elev. m a.s.l.			Routine	alert
WL	LS	LEVEL STAFF	Dam upstream face	(*)	(*)	reservoir elevation manual reading		Weekly manually whenever LR is not in function	Twice daily
	LR	Water LEVEL RECORDER	Dam upstream face	(*)	(*)	reservoir elevation digital reading and registration		Daily	Twice daily
Pz	Ex	PIEZOMETER external to the dam body	downstream of the dam and of the reservoir	(*)	(*)	registration of deep aquifer and groundwater levels	they are fed by batteries, which functioning shall be periodically checked.	Daily automatically; monthly manual	Twice daily
	B	PIEZOMETER inside dam body (fixed) installation	Dam foundation	(*)	(*)	registration of water pressures at defined elevation below dam		Daily automatically; monthly manual	Twicedaily
DF (2) (4) (5)	V	V NOTCHES	inside dam galleries (and wherever applicable)	(*)	(*)	measures of amount of leakages through the dam and through the dam foundations. Measures of leakages through the rockmass around the Power Waterways.	The measurements shall be carried out and presented separately for drains attaining the dam upstream face (included dam joints drains) and drains crossing the dam foundations, in turn divided in bottom, right abutment and left abutment zones.. (4)	Monthly, or in any case every 10m of reservoir level raising.	Daily
	D	WATER LEVEL INDICATOR (data logger)	at pits in correspondence of dam galleries exit and in dam bottom gallery pump pits	(*)	(*)	measures of levels to quantify the amount of leakages through the dam and through the dam foundations	see above.	Daily	Every hour

Initials		Instruments Type	Position			Scope	Notes	Monitoring schedule (1)	
			Area	Ref. dwg	Elev. m a.s.l.			routine	alert
	L	WATER LEVEL GAUGE STAFF at galleries exits.	at pits in correspondence of dam galleries exits	(*)	(*)	measures of levels to quantify the amount of leakages through the dam and through the dam foundations	see above.	Monthly, or in any case every 10m of reservoir level raising.	Daily
	P	DRAIN FLOW measurement by means of PUMPS at bottom galleries pits	at dam galleries exit	(*)	(*)	measures of leakages through the dam and dam foundations recollected below tailrace water level	the operating criteria and conditions and alarm system for the dam drain pumping system is to be automatically implemented, to be integrated with pumps supplier operation manual. (4) (5)	Daily automatically or manually (with Staff) in case the automatic system is out of operation.	Twice daily
T	TC	THERMOCOUPLES	inside the dam body	(*)	(*)	registration of punctual temperature inside dam body.		Montly	Daily
	FO	FIBER OPTIC CABLES	inside the dam body	(*)	(*)	continuous registration of temperature inside dam body.		Monthly	Twice daily
DS (6)	CI	COLLIMATORS	Internal: inside dam galleries alignments. External: alignments on dam crest and on d/s face	(*)	(*)	check of dam displacements (overall dam displacement or single block movements)		Monthly	Daily

Initials		Instruments Type	Position			Scope	Notes	Monitoring schedule (1)	
			Area	Ref. dwg	Elev. m a.s.l.			routine	alert
DS (6)	Fa Ma	FIXED AIM and MOBILE AIM	inside dam galleries and on dam crest	(*)	(*)	see above	to be used for displacements check together with collimator basements and mobile aims.	see above	Daily
	Jm	JOINT DEFORMOMETER automatic and manual types	inside dam galleries and on dam crest	(*)	(*)	check of joint movements		Daily	Daily
	PI	Inverted PENDULUM	On specific dam Instrumented Sections (IS)	(*)	(*)	Registration of relative displacement between dam and foundation.	Automatic readings by the Automatic Fixed Coordinometer	Daily automatic; monthly manual	Twice daily
	PD	Direct PENDULUM	On specific dam Instrumented Sections (IS)	(*)	(*)	registration of horizontal displacements at several elevations along the IS dam block.	Automatic readings by the Automatic Fixed Coordinometer	Daily automatic; monthly manual	Twice daily
E	EB	EXTENSOMETERS in boreholes	On specific dam Instrumented Sections (IS)	(*)	(*)	registration of foundation deformations in respect to the dam structure		Daily automatic	Daily
WL	MS	Meteo Station	(*)	(*)	(*)	Meteo data (temperature, wind, rain) records	Data need to be discharged from the instrument and elaborated on monthly basis.	Data discharge and elaboration on monthly basis	Weekly
	LR	Water levels Radar recorded	(*)	(*)	(*)	River level measurement		Data discharge and elaboration on monthly basis	Weekly

Initials		Instruments Type	Position			Scope	Notes	Monitoring schedule (1)	
			Area	Ref. dwg	Elev. m a.s.l.			routine	alert
WL	LS	WATER LEVEL STAFF at Dam downstream face	Dam downstream face	(*)	(*)	River level of plunge pool	Measures to be correlated with radar levels and turbine outputs	Manually on daily basis.	Twice daily
	AC	Accelerographs	Dam, inside galleries at location to be defined (*)	(*)	(*)	Seismic movements	Measures to be considered and examined only in case of seism. Recorded automatically.	Continuously in automatic, until seismic event is recorded.	Continuously in case of seism

(1) These are the frequencies of measuring activities.

(2) Flow-meters, manometers, drain pipes extensions, other tools and adaptors are at hand at Dam control building, available for spot measurements where occasionally required on single drains or piezometers holes registering important leaks.

(3) For DS instruments all reading devices equipment shall be at hand at Dam control building.

(4) Flow measurement of individual drains that are discharging important leaks shall be monitored with same criteria of the general monitoring of drainage system of the Dam.

(5) Drains recollecting warm water springs in foundations, if any, shall be possibly identified and monitored as separate datum also.

(6) Reading devices equipment shall remain at hand at Dam control building.

(*) To be filled at detailed design and construction stage.

Table 1 - SUMMARY TABLE of Instruments CONTROL PLAN

B.2.3 INSTRUMENTS MEASUREMENTS REPORTING

The measuring activity (that can be commenced during the impounding period) shall continue during the period of operation of the plant.
All data gathered shall be always available on site. A REPORT presenting all data gathered by the instruments shall be prepared on MONTHLY basis.

Several data of instruments installed in the dam body can be extracted from the synoptic panel of the Acquisition Unit that is assumed will be provided to gather all data at Dam Control Building, without need to inspect the single instruments unless for specific punctual verifications. The Acquisition Unit is assumed provided with dedicated software that allow selection and extraction of data in tabular and graphical format and combination of readings of different instruments at same time.

The report shall include and organize the data and information described for each type of measurements. The data required to be presented are listed along this chapter for each type of instrument, to be provided in tabular and graph format.

Typical graphical representations shall be provided during monitoring for the most relevant type of records, where necessary combined with or referred to the reservoir water levels, plant operation data or rain data, and also referred to relevant design reference graphs when available (for instance for dam uplift), to allow a prompt evaluation of the results, and consequently definition of actions to be taken if required.
The following indicative list of data to be presented is provided for reference:

1) TEMPERATURE
    a. Thermocouple: reading & charts
    b. Optic Fiber: reading & charts
2) DRAIN FLOW
    a. Piezometer inside Dam body: reading & charts
    b. Piezometer outside Dam body: reading & charts
    c. Dam Instrumented Sections: Piezometers Water level
    d. Drain: water flow monitoring
    e. V-notch: water flow monitoring
3) LEVEL
    a. River, impounding and RCC levels: reading & charts
    b. Plunge Pool water level readings
4) STRESS
    a. Multi-Points Extensometers: reading & charts
    b. Single-Point Extensometers: reading & charts

c. Mechanical Strain Gauges: reading & charts

5) DISPLACEMENT
   a. Inverted Pendulum: reading & charts
   b. Direct Pendulum: reading & charts
   c. Dam absolute movements: from inverted and direct pendula data elaboration
   d. Joint Deformometer (3D tern manual): reading & charts
   e. Joint Deformometer (3D tern automatic): reading & charts
   f. Collimator alignment: reading & charts
6) VARIOUS
   a. Weather report: meteo station data & charts
   b. Rainfall historical sequences: reading & charts

The report in ROUTINE or ALERT conditions (see par. 1B.3.2 CRITERIA FOR ROUTINE OR ALERT PROCEDURE APPLICATION also next chapter) shall be forwarded MONTHLY to the contacts reported in a table constructed as follows:

Plant Management Structure				
Name	Position	Telephone	E-mail	Mail address

Table 2 - Typical table of Contacts for Routine or Alert procedure implementation

For both conditions this task is a responsibility to be assigned to the manager of the PMS.

In case of need to trigger an ALARM condition, or in case of need to implement the CONTINGENCY PLAN, described in Part E of the Dam Safety Plan, the contacts reported in par. E.7.4 of the Dam Safety Plan shall be immediately contacted.

The table above as well as the tables reported in par. E.7.4 of the Dam Safety Plan shall be filled by all the parties within the Owner organization involved in the Plant operation.

B.2.4 METEO and RIVER LEVELS MEASUREMENTS

The Meteo station is foreseen to be installed near dam and shall continue to record the meteo data (rain, wind, temperatures) from the automatic acquisition unit on monthly basis, for correlation with other measurements, and finally presented in a yearly report (possibly without interruption since the construction period).
Readings of river levels downstream of the dam shall be plotted on the river rating curve to obtain flow data. This rating curve shall be updated (during the construction and impounding period, as well during the operation period) by calibration with direct measures of flow in both dry and wet season conditions.

Levels records (in automatic downstream of river diversion) shall be taken on daily basis.
Levels and flow data shall be gathered and presented in a monthly report, correlated to the Power House, Spillway, Middle Outlets and Ecological discharge valves outflows (as far as it is the case).

B.2.5 RESERVOIR WATER LEVEL MEASUREMENTS

The reservoir water levels measurements shall be carried out by means of automatic (Level indicators) and manual (graduated level staffs) instruments.

During the operation of the plant the automatic level indicators will record water levels in the reservoir, and will transmit the data to the acquisition unit system, that is supposed to be present to gather the results at Dam Control Building. Whenever the automatic acquisition does not work, manual and visual readings of the reservoir water levels on the staffs that are installed along the dam upstream face shall be carried out.

The levels records shall be available immediately on site for all the purposes requiring decisions or actions linked to the reservoir water levels.
The plot of the river levels shall be always included in the monthly reports associated to all the other instruments measurements, for correlation purposes.

B.2.6 OUTFLOWS AT POWER HOUSES

This applies for each Power House.
The Power House Outflows are automatically continuously measured on each unit outlet and results gathered at Power House control building.
This measurement is firstly used in automatic by the control system of the plant for the energy production and for the management of each of the unit.
The daily records of the overall flow discharged at Power House shall be also gathered and reported in the monthly report of the instruments monitoring for comparison (also with tailwater levels) and overall understanding purpose.

Together with this datum, whenever the Middle Outlets and/or Spillway is in function, the relevant outflow, derived by the use of the reservoir level measurements (see previous paragraph) and by the relevant rating curves, shall be also provided in the monthly report of the instruments monitoring.

B.2.7 WATERS MONITORING: SPRINGS

The existing natural springs, if any, are required to be identified by:

- position
- water flow
- water temperature

A regular record (possibly on monthly basis) of water flow for the springs shall be carried out. Data shall be presented correlated to the river and reservoir levels and to the rain records, as measured in the closest available meteo-station.

The data shall be presented in graph and tabular format in the monthly report.

B.2.8 WATERS MONITORING: DAM PIEZOMETERS

A) PIEZOMETERS INSIDE THE DAM BODY

The piezometers inside the dam body are to be monitored automatically by the Acquisition Unit that is supposed to be present to convey the data at Dam Control Building.

Measures of pressures (water levels) at each piezometer shall be done daily in automatic and manually on monthly basis. This frequency can be modified in occasion of important rain events, rapid raising/lowering of the reservoir, as well as in case of seism.

Measures of drain discharge shall be plotted versus the time on monthly basis, and presented in the monthly report, correlated for comparison purposes with other following outputs:

- foundation drains flow;
- dam drains flow;
- river levels;
- dam piezometers readings;
- readings of other piezometers located in the Batoka site area (see next paragraphs);
- rain records at the meteo-station;

- reservoir levels.

The data shall be presented as far as possible grouped along different alignments with water levels plotted on transversal and longitudinal dam vertical sections to visualize correlations and trends along the dam foundation. Measurements from external piezometers (see next paragraph) aligned along these sections shall be also shown on the same drawing.

B) PIEZOMETERS OUTSIDE THE DAM BODY

The water table in the flanks of the river valley below the maximum reservoir level downstream of the dam shall be monitored also by means of external piezometers, whose number and position will be defined during detailed design and construction stage and reported in a table constructed as follows:

No	ID	Length	Easting	Northing	Elevation	Type	Position
		m	m	m	m.a.s.l.		
1							
2							
...							

Table 3 - Typical table with positions of piezometers

The monitoring shall be regular (daily measurement in automatic, on monthly basis manually) and shall cover dry and wet season period.
Every month the measurements shall be collected by means of portable reading units and provided in graphical and tabular format, together with all the other information pertinent to the meteorological conditions and dam as indicated in the previous paragraph.

B.2.9 DAM DRAIN MONITORING

Monitoring of dam galleries conditions and relevant drainage system is very important to be carried out during the plant operation period.

The amount of water coming out from the Dam drain gallery shall be monitored by means of the water level sensors (divers) and V-Notches purposely foreseen.

The overall amount of water discharged at each gallery level shall be also measured at each gallery exit, where automatic (with diver in a dedicated pit) and manual (with V-notch, meter, or pit of known volume) measurements can be made.

For the galleries under tailrace level the same measure shall be taken recording the water levels in the pit and the pumps operation records.

Samples of the drain water, flowing out of the drainage pipes crossing the dam foundation, must be also taken in order to check the turbidity.
Frequency of turbidity test will be revised on the basis of flow measurements. At any significant increase of seepage, it will correspond an increase of test frequency.

The measurements shall be carried out and presented separately for drains attaining to the dam upstream face (see also prescriptions of next paragraph), the dam joints drains, and drains crossing the dam foundations, in turn divided in bottom, right abutment and left abutment zones.

Calibration of V-notch will be checked by means of a graduated bucket and a stopwatch.

In case of necessity (when there is evidence of significant or anomalous outflow from a single drain), measurements of single drains outflow can be carried out manually with a bucket after installing a bent pipe extending the drain pipe, as foreseen in design documents.
Adaptors, pipes extensions, flow-meters and manometers shall be available at Dam control building store to be used for spot measurements on single drains when needed.

The following additional aspects shall be considered in organizing such activity:

1. Consideration should be given to monitoring the depth and type of sediment at the gallery weirs before cleaning each gutter, rather than just removing and discarding it. This shall be done on monthly basis.
   Samples of the sediment should be analysed on occasion for grain size, plasticity, pH, and composition. This information can be very useful if any piping or erosion starts in any of the foundation drains. The pH can be used to indicate if some of the seepage is coming from internal drainage of the dam itself, picking up calcium hydroxide (basic) on the way. Consideration should also be given to using a hand held "pistol" type temperature indicator to check the temperature of water coming from foundation drains. Rather than rely only in keeping complex records that are not always readily available, a simple method of documentation can be also used to just write the date and temperature on the wall next to the drain with a pencil. If the temperature changes, the date and new temperature should be noted

2. Consideration should be given to recording the volume of water pumped per week or month from the gallery sumps. Seepage flows shall be measured using weirs in the gallery gutters, but the pump records can also be used as a check. Also, it is possible on very high dams to observe a

sudden increase in gallery flows sometime during the life of the structure due to a failed waterstop, a sudden crack, or a significant change in foundation seepage, that could flood the gallery gutter and weirs. In this case, checking the pumping records (time and capacity, or by a flow meter) can provide an idea of the amount of flow.

Monitoring of dam galleries drainage system shall be carried out monthly in routine conditions, as far as the manual readings are concerned.
Water levels indicators (data loggers) shall be recorded in continuous the levels in the pits at the exit of the galleries, and send the records to the control center.
The pumps pits can be equipped with a system of recording and progressive starting of pumps (details to be defined with the supplier)

All the records shall be presented on a monthly report.
Data shall be presented correlated to the impounding levels and with the rain records as measured in the meteo-station.

Graphs of following outputs (related to water measurements) shall be provided in the same plot, for comparison purposes:

- Foundation drains
- Dam drains
- Reservoir levels
- Springs drains
- Dam piezometers
- Readings of other piezometers located in the site area
- Rain records at the meteo station.
- Power Houses operation (turbines in function, discharged flow).

B.2.10 THERMOCOUPLES

At feasibility design stage, thermocouples are foreseen to be installed in the dam.

The output of the thermocouples installed during the dam construction shall be acquired (as far as possible) on monthly basis.

The results shall be plotted versus time on monthly basis, with indication of the location and RCC zoning characteristics at that location (placing temperature and time, cement content, etc.).

The results shall be compared with the expected trend of temperatures in the dam body as defined in the next chapter.

B.2.11 FIBER OPTIC CABLES

At feasibility design stage, fiber optic cables are foreseen to be installed in the dam. The prescriptions for the fiber optic sensors readings are recalled hereinafter.

The output of the installed fiber optic cable sensors (FBG) shall automatically acquired in routine conditions on monthly basis.

The results shall be plotted versus time on monthly basis, with indication of the RCC characteristics (placing temperature and time, cement content, etc.).

Graphs of thermocouples and fiber optic sensors output shall be provided in the same plot, for comparison purposes, and compared with the expected trend of temperatures in the dam body defined in the next chapter.

B.2.12 EXTERNAL BENCHMARKS

External benchmarks are foreseen to be available around the dam site. They will be used during construction and they are not object of systematic monitoring during the operation period, however the Operator shall know their existence for any willingness of re-calibrating at large scale existing monitoring system basepoints (for instance after a seism, if it is deemed necessary to re-check the position of the collimators basepoints on the dam abutments, or the one on the dam crest, in respect to points more distant from the dam structure).
The position of available external benchmarks shall be provided in a dedicated drawing before plant enters in operation.

B.2.13 COLLIMATORS WITH FIXED AND MOBILE AIM

Fixed and mobile aims with collimator are foreseen to be installed in the dam.
Measurements shall be carried out manually by means of an optical collimator and fix and mobile aims by a skilled surveyor, registering horizontal and vertical displacement on each aims alignment and for each aim (typically one for each dam block).

Measurements shall be done for routine conditions on monthly basis, or in case of sudden important change of levels occurred in the reservoir.

Records shall be presented on a monthly report, recorded and plotted on the longitudinal section where they are measured and on the cross sections of the Dam, together with the reservoir water levels and the following instruments measures (described in the next paragraphs):

- External Targets displacements
- Invert and direct pendula
- Extensometers
- Joint deformometers

The measurements must be always reported separately for each settlement-measuring device.
The results shall be assessed as described in the next chapter.

B.2.14 JOINT DEFORMOMETERS

Joint deformometers are foreseen to be installed inside the inspection and drainage galleries of the dam.
Joints deformometers automatic readings shall be carried out, registering horizontal and vertical displacement on each joint, with the Acquisition Unit, and manually with Portable reading units whenever a defect or interruption or anomalous trend is observed in the automatic readings.

Measurements shall be done on monthly basis, or in any case every 10m of sudden reservoir level raising.
Results shall be provided in a monthly report, organized and presented as done for the external targets and collimators displacements (see previous paragraphs).

B.2.15 PENDULA

The INVERT pendula are foreseen to be installed with the head mounted in the galleries above tailrace water level.
The DIRECT pendula will be installed in the dedicated shafts.

All pendulum measurement record is foreseen to be daily and automatic.
A monthly basis check by manual reading is by default required.

Measurements shall be recollected on monthly basis, recorded and plotted versus the time.
Records shall be provided, together with other displacements and deformation measures, plotted along the dam sections pertinent to the pendula, for an immediate comparison with the expected values of settlements (see next chapter).

B.2.16 EXTENSOMETERS

Extensometers are foreseen to be installed in the dam.
Extensometers automatic readings shall be carried out with the Acquisition Unit, and manually with Portable reading units whenever a defect or interruption or anomalous trend is observed in the automatic readings.

Measurements of extensometers shall be recollected on monthly basis (default automatic readings).
Results shall be provided in a monthly report, recorded and plotted versus the time with indication of the reservoir water levels, correlated to the other displacement measures (data available form external targets, collimators, pendula) for an immediate check, as described in the next chapter.

B.2.17 ACCELEROGRAPHS

Strong motion measurement record is continuous and automatic in case of seismic event. The Acquisition Unit is typically set to also register such measurements.
Measurements, whenever acquired by the instrument, shall be recorded and plotted versus the time and presented together with the other dam instrumentation measurements.

Any exceptional or anomalous record shall be transmitted as soon as possible to the National Authorities deputed to the seismic events monitoring of the region, and they shall be made available for comparison, for study and evaluation purpose, with the accelerographs of the seismic cases used for the dam stability verifications.

In case of seismic event, further specific measures for structural control of specific parts of the works will be decided on case by case basis.

B.2.18 DOWNSTREAM WORKS INSPECTION

The inspection of the works and the downstream areas aim to find and measures any signal of potential leakage or static problems to the structures connected with the operation of the Plant.

A careful visual inspection of the works should be carried out with particular attention to the hydraulic steel structures when they operate and Dam upstream zone (as far as possible to inspect it from upstream and from the galleries).

The inspection of the works (object, methodology, frequency, timing and reporting requirements) is described in more detail in Part D of the Dam Safety Plan, dedicated to the Plant maintenance, to which reference shall be made.

B.3 DAM MONITORING RESULTS ELABORATION: ROUTINE AND ALERT CONDITIONS

B.3.1 INTRODUCTION

This chapter:

- illustrates the principles of how to evaluate and assess the results of the measurements carried out with the instruments,
- consequently individuate the conditions to implement the procedures associated to the ROUTINE or ALERT and, in case, to the DRAWDOWN conditions.
- Describes the ROUTINE, ALERT, CONTROLLED DRAWDOWN and EMERGENCY DRAWDOWN procedures.

During the normal operation of the plant, within the PMS, an instrumentation monitoring reporting shall be organized and continued as done in first impounding period.
For some kind of measurements, there is not a fixed *a priori* specific threshold that marks the passage from routine to alert condition. In any case it is necessary to define or adjust threshold values after first (and subsequent) period of impounding and operation according to the first (and subsequent) measures, as well as the first assessment of the dam behavior and of any anomalous record of any instruments carried out during the first impounding period.

A general criterion for the application or the Routine or Alert procedure is in any case generally fixed as follows:

- When the reservoir level is stable (not raising or lowering), the alert procedure is activated if there is a trend of increasing with time of the datum recorded.
- When the reservoir level is raising or lowering, the alert procedure is activated if there is a trend of increasing of the datum recorded that is time dependant and not level dependant.

The assessment is focused and organized in this chapter on the following main aspects dealing with the dam stability and safety:

1. UPLIFT PRESSURES ON DAM FOUNDATIONS
2. GROUNDWATER CIRCULATION
3. LEAKAGES
4. DAM FOUNDATION SETTLEMENTS and DAM DISPLACEMENTS
5. DAM THERMAL CONDITIONS

The table at par. B.3.2 CRITERIA FOR ROUTINE OR ALERT PROCEDURE APPLICATION summarises the criterion of Routine or Alert procedure application. It shall be updated and detailed during detailed design and construction and first impounding phases.

B.3.2 CRITERIA FOR ROUTINE OR ALERT PROCEDURE APPLICATION

In order to evaluate the Dam behavior the criteria stated in the following tables shall be followed, for the most important parameters to be controlled, namely:

1. Dam uplift pressures and groundwater
2. Dam and Power Waterways leakages

Other parameters that shall be controlled are:

3. Dam foundation settlements
4. Dam displacements
5. Dam RCC temperatures

Such parameters will be controlled by means of some of the instruments available, as described in previous chapter 1 "
DAM INSTRUMENTATION OPERATION AND MONITORING", and assessed as described in this chapter.
The most critical parameters are the ones at points 1 and 2 above, that shall be firstly checked in respect of possible triggering of alert procedures.

Records of instruments devoted to the control of the other parameters will be collected systematically, but used only whenever necessary to analyze the possible causes and effects in case an unforeseen event or anomalous trend is found.

In the following tables, the threshold and acceptable limits, where indicated, are the ones defined in the subsequent paragraphs of this chapter.

STABLE WATER LEVEL IN THE RESERVOIR		
	Routine Procedure (1)	Alert Procedure (2)
UPLIFT PRESSURES ON DAM FOUNDATIONS	Within the threshold limit defined in the detailed design of the dam (see par. B.3.3 below). Stable	Exceeding threshold limit defined in the detailed design of the dam (see par. B.3.3 below). Increasing with time, independently from season trend.
GROUNDWATER	Stable or following season trend.	Increasing with time, independently from season trend.
LEAKAGES THROUGH DAM FOUNDATIONS, DAM BODY, POWER TUNNELS	Stable or (for foundation drains) variable according to rain/dry season trends.	Increasing with time (for foundation drains only if independently from season trend). For local drains when jetting continuously.
DAM FOUNDATION SETTLEMENTS	Stable.	Increasing with time.
DAM DISPLACEMENTS	Stable.	Increasing with time.
RCC DAM TEMPERATURES	Within acceptable limits fixed by the detailed design of the dam	Increasing above acceptable limits fixed by the detailed design of the dam

Table 4 - Criteria for Routine or Alert procedure application – case of stable reservoir water level

CONTROLLED RISING OF WATER LEVEL IN THE RESERVOIR		
	Routine Procedure (1)	Alert Procedure (2)
UPLIFT PRESSURES ON DAM FOUNDATIONS	Level dependant increasing with ratio <1. In any case within the threshold limit defined in the detailed design of the dam (see par. B.3.3 below).	Time dependant increasing. Above the threshold limit defined in the detailed design of the dam (see par. B.3.3 below).
GROUNDWATER	Level dependant increasing	Time dependant increasing.
LEAKAGES THROUGH DAM FOUNDATIONS, DAM BODY, POWER TUNNELS	Level and (for foundation drains) season dependant increasing	Time dependant increasing. For local drains when jetting continuously.
DAM FOUNDATION SETTLEMENTS	Level dependant increasing.	Time dependant increasing.
DAM DISPLACEMENTS	Level dependant increasing.	Time dependant increasing.
RCC DAM TEMPERATURES	Within acceptable limits fixed by the detailed design of the dam	Increasing above acceptable limits fixed by the detailed design of the dam

(1) for the application of the ROUTINE PROCEDURE all conditions stated in the table shall be verified;
(2) the ALERT PROCEDURE will be activated as soon as one of the conditions will be verified, or the Reservoir water level is higher than maximum operating level.

Table 5 - Criteria for Routine or Alert procedure application – case of raising reservoir water level

If above the Spillway sill level uncontrolled reservoir filling occurs due to an exceptional flood event that do not allows to maintain the water level in the reservoir within the maximum operating level:

- alert procedure will be firstly activated;
- then the procedure "OPERATION CRITERIA IN CASE OF EXCEPTIONAL HIGH FLOODS (RESERVOIR ROUTING) described in Part E of the Dam Safety Plan will be implemented;
- then, as far as necessary, the Emergency drawdown procedure described in this and in Part E of the Dam Safety Plan can be activated.

If according to the results of observations carried out during the Alert Procedure, an emergency state is triggered, the Controlled Drawdown Procedure or the Emergency Drawdown Procedure will be activated, as described in last paragraphs of this section.

B.3.3 UPLIFT PRESSURES ON DAM FOUNDATIONS

The levels registered in the piezometers inside the dam body intercepting the dam foundations, correlated with the upstream and downstream water levels reached during the impounding, shall be compared with the value of uplift pressure assumed in the dam stability calculations at the position and depth of the piezometer itself, scaled in proportion to the reservoir water level.

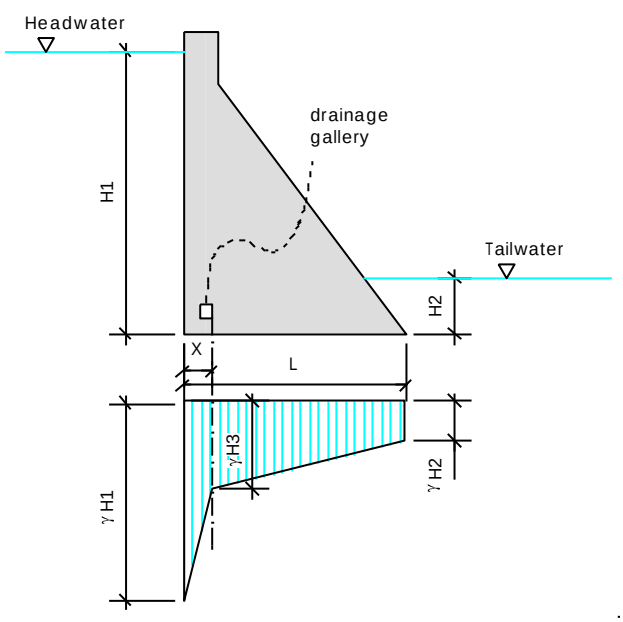
In principle, wherever the observed levels in the piezometers exceed the values assumed in the design, the ALERT procedure shall be activated, unless for minor deviations or local anomalies that can be imputed to local geological features or specific reasons, that can be evaluated on case by case basis.

In sake of simplicity for an easy first appraisal of the phenomenon the comparison can be made on the instrumented sections, in respect to the assumptions made on design uplift water levels for the Normal Operating Conditions in the Dam stability analysis that will be developed during the detailed design, from which the following figure represent a typical example.

Uplift pressures considered varying from the hydrostatic pressure relative to US Army Corps of Engineers, "Gravity dam design", EM 1110-2-2200, 1995:

- *at u/s toe* *H1 =the reservoir water level*
- *at d/s toe* *H2 = tailwater level*
- *at drainages* *H3= 0.33 x (H1-H2) x (L-X)/L + H2*

as illustrated in the following figure:



L = dam base length

X = drain distance from u/s toe

H2= tailwater depth

The readings of the piezometers shall be plotted and compared with the uplift design reference line (in the figure above shown in blue, as example) This kind of outputs shall be provided for all instrumented sections, to easily individuate any anomalous value exceeding the assumptions made in the dam design.

It's also important to observe the pressure increase versus reservoir level for the various piezometers; data shall be presented showing the water level in the piezometers installed at different galleries versus the time and the reservoir water level. If deemed necessary, correlation curves (H piezometers versus reservoir water level) may be drawn.

It is recommended that before operating the plant the present document be integrated with a table in which for each piezometer there are provided the following values of total head (expressed in meters above sea level) used for the purpose of calibration of the instruments:

1. ALERT TRESHOLD
   This is the value above which the monitoring of the piezometer shall pass from ROUTINE to ALERT conditions (see next dedicated paragraphs)

2. ALARM TRESHOLD
   This is the value above which an alarm shall be activated, being the head reaching about 90% of the value assumed in the design for the stability of the dam.

Of course in such case an analysis and check shall be made on the specific piezometer also in relation to the trends of other and adjacent piezometers.

Piezometers can be dependent from reservoir water elevation. For this reason the above threshold values shall not be univocally defined, but instead given as function of reservoir water level (H).

The software of the instruments acquisition unit can be already programmed to have inserted the following values, as function of H, for the automatic monitoring of the piezometers levels.

PIEZOMETERS typical threshold values		
Piezometer ID	1= ALERT TRESHOLD	2=ALARM TRESHOLD
-	m a.s.l.	m a.s.l.
1	>z1 +k1 * (H- z1)	> z1+k1′ * (H- z1)
2	> z2 +k2 * (H- z2)	> z2 +k2′ *(H- z2)

Table 6 – Piezometers typical threshold values

B.3.4 GROUNDWATER CIRCULATION

The monitoring activities of the groundwater is aimed at detecting potential anomalous or local unexpected trend of pressure or water flows increasing in connection to the progress of impounding or reservoir fluctuations.
The analysis will be made through the combined assessment of measurements of external waters (rain, river flows, springs) and the deep piezometers available. A table like the following shall be prepared with all the available piezometers listed, and the ones possibly intercepting any aquifer highlighted.

Piezometer	location	Top elevation	Bottom elevation	Notes
1	…	…	…	…
…	…	…	…	…

Table 7 – Typical list of piezometers

Typical outputs of the monitoring to be provided are plots of deep aquifer piezometers level versus time and versus impounding water level, allowing to check the criteria for routine or alert procedure application.

Anomalous records of leakages from the drains, among the ones listed in the table above, intercepting the dam foundation in correspondence of possible special or weak zones, shall be linked and correlated to the above observations.

All the above is mainly related to the dam structure, but at the same time useful information about the groundwater circulation and its dependence from the reservoir water level shall be acquired by the measurements available from the available drainage system/ other external drainages on all the Batoka site.

A table analogue to the one envisaged for the Dam piezometers shall be prepared, in which for each piezometer there are provided the following values of total head (expressed in meters above sea level) used for the purpose of calibration of the instruments:

1. ALERT TRESHOLD
   This is the value above which the monitoring of the piezometer shall pass from ROUTINE to ALERT conditions.

2. ALARM TRESHOLD
   This is the value above which an alarm shall be activated, in the case the deep piezometers level gets close to the limit compatible with the dam or other structure stability (according to the detailed design assumptions).
   Of course in such case an analysis and check shall be made on the specific piezometer also in relation to the trends of other and adjacent piezometers and on the reservoir.

Also these piezometers can be dependent from reservoir water elevation. For this reason the above threshold values are not univocally defined, but shall instead be given as function of reservoir water level (H).

Also in this case, the software of the instruments acquisition unit can be already programmed to have inserted the following values, as function of H, for the automatic monitoring of the piezometers levels.

As far as groundwater circulations (deep aquifers) no direct measures of the total seepage flow are supposed to be available and the influence of the groundwater, the surface water flow, of the precipitation and of the impounding process or reservoir fluctuations on the seepage flow is quantitatively unpredictable, making definition of sharp seepage threshold values unreliable.

As far as the flow is concerned, indicatively, the total amount of recollected water from these drains should remain in the range of some liters per second which, in terms of volume, cannot be considered critical at the scale of the project.
The temperature of the drained water shall be measured, to gain information about its origin.

B.3.5 LEAKAGES

LEAKAGE (SEEPAGE) THROUGH DAM FOUNDATIONS

The measurements gathered from the drains crossing the dam foundations will be analyzed considering the sum of contribution of all the drains crossing the dam between two subsequent galleries, and where necessary correlated to specific ground or foundation features or local weak zones.
As far as possible the total flow value sum of contribution of all the drains catching possible springs on dam foundation shall be recorded and assessed separately from other drains or water outputs.

Indicative threshold value of leakage through dam foundations will be provided after observations relevant to the first year of impounding and the specificity of the dam foundations, the flows will be monitored and any anomaly at local scale of a single drain as well as distributed or total leakage flows will be recorded and assessed. As far as the impounding and plant operation go on and further data and information are gathered and interpreted, leakage flow alert threshold values will be updated or refined, if necessary.
In any case, the monitoring of the leakage trends (that in normal conditions shall follow the reservoir level trends and generally be stable in absence of variation of the reservoir levels) is much more important than the absolute value of recorded leaks.

Whenever water is clearly flowing (and not dripping or oozing) from a specific drain or point, such drain shall be object of monitoring with alert procedure.

LEAKAGE THROUGH THE RCC DAM

Any water incoming in the dam body from RCC dam drains or from vertical joints drains shall be object of local monitoring.
Whenever water is clearly flowing (and not dripping or oozing) from a specific drain or point, such drain shall be object of monitoring with alert procedure.

As far as the overall flow measured at the exit of the drain galleries, threshold values can be established if needed during the operation of the plant, taking into account the monitoring results of the prior phases and on their interpretation.

The extreme limit threshold value of the total flow recollected at lowest gallery can be limited to the overall capacity of the pumps that will be installed (this applies also for the flow coming from drains crossing the foundations). The pumps in the pits will be set with a progressive water level based functioning and alarm system. If the maximum capacity flow is exceeded the contingency plan shall be triggered.

Typical outputs of leaks monitoring relevant to both leakages through foundations and through RCC dam are graphs reporting the amount of leaks versus the time, presenting the distinct contributions of V-notches and single drains and their location and type, allowing an immediate understanding of the correlation with the reservoir fluctuations and detection of possible local or generalized anomalous trends.
Additionally tabular outputs shall be also gathered with indication of water temperature, where measured (especially in drains through foundation).

B.3.6 FOUNDATION SETTLEMENTS and DAM DISPLACEMENTS

The dam foundation settlements, measured with the extensometers foreseen to be installed in boreholes and with the collimators along the dam downstream face, are assumed will be compared with the expected deformations resulting from the dam stability analysis at design level for the typical (calculation) sections and for different reservoir levels.

The comparison will be conducted using the nearest calculated section available, considering the scenario closest to the reservoir level at the moment of the measurement.
The assessment will be conducted considering, with engineering judgment, all possible causes of any movement recorded, and the levels of the impounding and of the dam at the moment of the measurements.

The same approach is foreseen for the dam overall displacements measured with direct and invert pendula and the relative displacements of dam single blocks or vertical joints measured by direct pendula or by collimators located on dam downstream face, that will be compared with the expected displacements resulting from the dam stability analysis carried out in the detailed design phase.

As a general indication, displacements and movements of few millimeters are considered negligible at the scale of the project and in consideration of the accuracy of the instrumentation used.

Typical outputs of the monitoring present displacements continuously plotted versus the time (in both upstream to downstream direction and left to right direction) and the same for pendula readings, together with reservoir elevations, allowing their interpretation and correlation.

Whenever significant anomaly, abrupt isolated displacements or a trend not following the rate of reservoir level fluctuations (ref. table at par. B.3.2 CRITERIA FOR ROUTINE OR ALERT PROCEDURE APPLICATION) and substantially not consistent with the assumptions made in the design will be detected, the alert procedure will be activated, and possible actions decided on case by case basis.

B.3.7 DAM THERMAL CONDITIONS

The readings of the thermal records provided by the thermocouples and fiber optic sensors in the dam body during the impounding shall be compared with the value of maximum temperatures assumed in the dam design thermal analysis to be carried out at detailed design stage at the position of the measurement.

To this purpose specific plots for temperature readings comparison are to be provided, for main sections and most representative scenario analyzed inn the detailed design.
Data extracted in tabular format from manual or automatic readings of both thermocouples and fiber optic sensors can be used to make such check. At the same time contours of temperatures can be plotted in significant sections and compared with the temperature filed assumed for design calculations, to detect possible macro zones of possible temperature deviation.

Whenever the observed temperatures reach values out of the designed range expectation, the ALERT procedure shall be activated.

B.3.8 ROUTINE PROCEDURE

During Routine procedure, instruments reading and reporting activities, are executed as described in this report (see in particular *Table 1* of par. B.2.2).

B.3.9 ALERT PROCEDURE

Instruments readings are incremented as follow:

- Daily readings will be carried out not less than twice per day (also during night time);
- Other readings will be carried out not less than daily.

More details are provided in see in particular *Table 1* of par. B.2.2.
The data, only pertinent to the instruments readings falling in the alert conditions, shall be promptly organized in reports for engineering evaluation.
If deemed necessary, according to the analysis of the results of observations carried out during the Alert Procedure, the Controlled Drawdown Procedure or the Emergency Drawdown Procedure will be activated.

B.3.10 CONTROLLED DRAWDOWN PROCEDURE

The Controlled Drawdown Procedure (CDP) is finalized to lowering the reservoir level without causing damages to the outlet structures such as Spillway, Power Waterways and Middle Outlets.

If according to the results of observations carried out during the Alert Procedure, an emergency state is triggered but any impending hazard is excluded, the Controlled Drawdown Procedure will be activated to perform inspections and repair works on the submerged structures as necessary and as far as possible (no dewatering is physically possible below the Middle Level Outlet minimum operating level).
This is valid only for a limited range of reservoir water levels and within the limits of capacity of the available discharging devices versus the incoming flows (therefore also depending on the season and on the duration of the emergency state).

In case it is required, for instance in order to inspect lower portions of dam upstream face or the portals of Power Tunnels, or for decisions consequent to the results of the instrumentation monitoring, to operate a reservoir drawdown aimed to maintain as much as possible the water level to the minimum possible level, the CONTROLLED DRAWDOWN PROCEDURE will be carried out in accordance to the instructions of the Contingency Plan provided in the Part E (Emergency Preparedness Plan) of the Dam Safety Plan.

It is to be noted that in exceptional conditions the Middle Outlets can be temporary work up to a exceptional minimum elevation, but it is to be reminded that maintenance operation in dry conditions in the reservoir and dam areas below this elevations are not physically possible.

B.3.11 EMERGENCY DRAWDOWN PROCEDURE

If the results of observations highlight an impending hazard for the dam safety, the Emergency Drawdown Procedure will be carried out in accordance to the instructions of the Contingency Plan described in the Part E (Emergency Preparedness Plan) of the Dam Safety Plan.

PART C - OPERATIONAL PLAN (Preliminary Plan)

C.1 INTRODUCTION

C.1.1 CONTENT AND STRUCTURE OF THIS PART

This is the PART C "OPERATION PLAN (Preliminary Plan)" of the Batoka Dam Safety Plan.

This part of the document, and its references, provides the feasibility design guidelines for the operation of the Batoka scheme, including the dam, power waterways, power house and other appurtenant structures.
It outlines the operation activities and procedures relevant to the Batoka Dam and Hydro Power Plants, to be detailed and implemented by the Project Management Structure that will be appointed by the Owner(s) of the Plant.
Being the design at Feasibility stage, this document has a Preliminary nature, being necessary to be developed and detailed during project implementation. The final plan is due prior to 6 months before initial filling of the reservoir according to the development and detail of the design, and further on according to the construction of the plant itself.

According to the structures of Ownership and Operation that Zambian and Zimbabwean countries and ZRA will put in place, the responsibility of operation of the dam and of the two plants can be assigned to different PMSs, that, in the frame of development of the manuals and of relevant implementation rules and structures, will organize (dividing and coordinating as appropriate) the work to be done and the limits and coordination of responsibilities. At the present stage of the design, and considering the importance to have an overall view of the Batoka scheme operation requirements, the document is developed as one manual for the whole scheme.

As far as the Batoka Plant OPERATION instructions are concerned, this part includes:

- The description and instructions for the OPERATION of the plant in normal condition of power production
- The instructions for the proper and safe operation of the hydraulic device and the use of the civil works.
- Other specific prescriptions for the use of civil works.

The instruction for measurements to be collected by the instruments available at Batoka, and for their presentation, use and assessment, are provided in Part B of the Dam Safety Plan.

This part of the report is divided in the following chapters:

1) INTRODUCTION
   Describes the content of the report and its structure.

2) ROLES and RESPONSABILITY

   Outlines the general frame of roles and responsibilities for the Batoka HEP operation and maintenance activities.

3) SYNTHESIS OF RESERVOIR AND PLANT OPERATING CONDITIONS

   In this chapter is provided a synthetic description of:

   - the plant operating rules and conditions,
   - the reservoir operating conditions,
   - the system of power supply feeding the Plant.

4) PLANT HYDRAULIC CONTROL DEVICES OPERATION

   In this chapter it is provided a description, for ordinary and exceptional cases of operation, of the opening and closure manoeuvres for each hydraulic control equipment (valves, gates, bulkheads, stoplogs) for all the hydraulic devices (Middle Outlets, Spillway, Power Waterways, Draft Tubes, etc.) of the Plant.

   The prescriptions for the environmental flows release are also reported, associated to the specific hydraulic devices devoted to the environmental releases.

5) HYDRAULIC CONTROL DEVICES RATING CURVES

   In this chapter it is mentioned that it will be provided a description of the hydraulic rating curve of each of the hydraulic devices of the Plant, including the reservoir rating curve.

6) OTHER PRESCRIPTIONS FOR CIVIL WORKS OPERATION

   In this chapter some specific prescriptions for the use of specific civil works at Batoka Plant are provided.

7) INSTRUCTIONS FOR PLANT OPERATORS FIRST TRAINING

   In this chapter there are guidelines and indications for the Operators, as well as a format for the spare parts check.

Drawings illustrating all the works are the ones of the Feasibility Design, separately issued.

C.2 ROLES and RESPONSIBILITY

C.2.1 LEGISLATIVE FRAMEWORK

The project has a transboundary nature, having the dam shared between Zambia and Zimbabwe.
The Zambesi River Authority (ZRA) is a transnational authority that deals with the Zambesi river, formed by the Council of Ministers of Zambia and Zimbabwe, the Board of Directors and the Executive Management.

The history of the Zambezi River Authority (ref. www.zaraho.org.zm) may be said to have begun in November 1964 when the Central African-Council appointed the Inter-Territorial Hydro-Electric Commission.
In May 1951 the Commission recommended the development of a dam at Kariba and hydro-electric power station. In June 1954 the Hydro-Electric Power Act was passed which provided for the establishment of the Federal Hydro-Electric Board charged with the function of Coordinating the generation and supply of electricity within the Federation.
In May 1956 the Federal Power Board was established pursuant to the enactment of the Electricity Act. This was a reconstitution of the Federal Hydro-Electric Board. The new Board was vested with the power to construct dams and power stations, to transmit electric power and sell same to Electricity undertakings. A hydrological data collection organization operating in each territory was also established.
In 1963, the Federation was dissolved. The integrated systems for the control of generation of power and its transmission continued to be operated and was fully developed as a single system under joint ownership and control of the two Governments of Northern and Southern Rhodesia under the Central African Power Corporation (CAPCO) which was established in the same year.
CAPCO was vested with the assets and liabilities of the Federal Power Board. The general function of CAPCO was to supply electricity to Electricity undertakings in the two territories while its conduct was regulated by a higher authority for power comprising two ministers appointed by each of the two Governments.
In 1987 the Zambezi River Authority Act was passed simultaneously in the two states of Zambia and Zimbabwe dissolving CAPCO and reconstituting it as Zambezi River Authority (ZRA).
CAPCO was divested of its electricity production and bulk distribution assets which were allocated to the National Electricity undertakings of the two states.
The ZRA was therefore left with the responsibility of the operation and maintenance of Kariba Dam Complex, investigation and development of new dam sites on the Zambezi River and analyzing and disseminating hydrological and environmental information pertaining to the Zambezi River and Lake Kariba.

ZRA is therefore assumed to be the dam Owner, intended as the organ charged to be responsible of the operation and maintenance of Batoka Dam.
The two national Plants linked to the dam can have different structures of Ownership and Operation, and that when will be appointed (by ZRA or by Zambia and Zimbabwe governments), will take the responsibility of the operation and maintenance of the Plant(s).

According to the structures of Ownership and Operation that Zambian and Zimbabwean countries and ZRA will put in place, the responsibility of maintenance of the dam and of the two plants can be assigned to different PMSs, that, in the frame of development of the manuals and of relevant implementation rules and structures, will organize (dividing and coordinating as appropriate) the work to be done and the limits and coordination of responsibilities. At the present stage of the design, and considering the importance to have an overall view of the Batoka scheme operation and maintenance requirements, the document is developed as one manual for the whole scheme.

It is assumed and recommended that the dam Owner (ZRA) is organized to be capable to manage, operate, maintain and protect the dam. In addition to this recommendation it should be noted that the dam Owner has liabilities to others under civil law.

C.2.2 MANAGEMENT STRUCTURE

The Owner of the Dam and of the Plants, within is structure or appointing external consultants, will build up one or more Plant Management Structure (PMS) that will be responsible of the management, operation and maintenance of the Dam and of the Plants. Such structure shall be such to cope with the need to properly operate and maintain the plant as described in parts C and D of the Dam Safety Plan, and to fulfill the reporting requirements described in Part D of the Dam Safety Plan.

C.2.3 SITE STAFF SKILLS AND TRAINING

The Batoka Dam and the two Power Houses are expected to be manned 24 hours per day.
Operation, inspection and maintenance of all aspects of the plant is to be undertaken by suitably qualified staff.
Training requirements related to operations of electrical and mechanical plant is described in dedicated section of this report. Training of staff is very important to achieve successful implementation of these Safety Plans.
Surveillance inspections are to be conducted by staff trained and certified as competent in dam safety inspections. Surveillance data assessments and dam safety decisions are required from qualified engineers experienced in dam safety management.

C.3 SYNTHESIS OF RESERVOIR AND PLANT OPERATING CONDITIONS

C.3.1 GENERAL

All that follows has to be revised and updated with the development of the detailed design and construction of the Plant.

The characteristics of the plant summarized below are referred to the current status of the feasibility design.

The main components of Batoka HES basically include:

- A dam, about 180 m high, hosting middle outlets and spillway with plunge pool
- no. 4 underground power waterways, two on each abutment, with intakes, tunnels, shafts, penstocks
- no. 2 outdoor power houses, one for each bank, for a total installed power of 2400 MW.

The dam body hosts on its central part a spillway divided into several independent channels controlled by gates, returning the floods into the river into a pre-excavated plunge pool in the river bed. Two middle outlets are also hosted into the dam body.

Each bank hosts No. 2 underground power waterways that including:

- Four intake structures, two on each abutment, all located close to the dam
- four power tunnels, two on each bank, about 600-700 m long each with a diameter of 9.5 m
- four high surge shafts and 200-300 m penstocks, ending into manifolds that feed the powerhouse.

The two outdoor powerhouses are located downstream of the pre-excavated plunge pool, currently about 450 m from the spillway lip. The powerhouse building is about 175 m long and 40 m wide, disposed quite parallel to the steep rock front and slightly rotated with the river alignment.

The two powerhouses host No. 12 turbines (No. 2x6x200 MW) obtaining a total installed capacity of 2400 MW. Six main step-up transformers are located in "open air" at the back of each Power House, on a dedicated deck.

C.3.2 PLANT OPERATING CONDITIONS AND FACILITIES DESCRIPTION

This chapter describes the operating conditions of the power plant in respect of:

- hydraulic constraints
- availability of those equipment which contributes to the minimum requirements of the safety of the power plant

For what the hydraulic conditions are concerned, the power plant is in normal operation whenever the following conditions occur:

- Reservoir water level (RWL) between Full Supply Level (FSL) and minimum operating level (MOL)
- generating units in operation
- no ecological flow released through the ecological discharge valve

An "exceptional" operating conditions is identified whenever the above conditions are not met, and in such case we can have different scenarios as indicated in following table.

CONDIITON	*Description*	*event occurrence*	*RWL, m a.s.l.*	*# units in op.*	*Q in each power tunnel, (max value in m^3/s)*	*MLO release*	*SPILLWAY release*
A	normal op	continuously	MOL ÷ FSL	12=(1 ÷ 6)+(1÷ 6)	Yes (411.2)	no	no
B	Controlled flood release	possibly yearly	< Spillway sill	12=(1 ÷ 6)+(1÷ 6)	Yes (411.2)	yes	no
C	Controlled flood release / flood event	possibly yearly	< FSL	12=(1 ÷ 6)+(1÷ 6)	Yes (411.2)	if needed (*)	if RWL > Spillway sill (*)
D	Flood event	possibly yearly	> FSL	12=(1 ÷ 6)+(1÷ 6) (if possible, (*))	>= (411.2) if plant operates (*)	if needed (*)	Yes (*)

Table 8 - Operation of Batoka - hydraulic condition

(*) Following OPERATION CRITERIA IN CASE OF EXCEPTIONAL FLOODS (RESERVOIR ROUTING) to be included at par. C.4.6.

For ordinary and exceptional cases of operation, the description of opening and closure manoeuvres for each of the hydraulic devices (valves, gates, bulkheads, stoplogs) of the Plant is preliminarily indicated and shall be provided in detail at further design and construction stages in the next chapter C.4 of this report and in the relevant manufacturer's specific operation and maintenance plans.

In the same chapter C.4 the environmental flows prescriptions are reported, associated to the specific hydraulic devices devoted to the environmental releases.

The reservoir rating curve, that shall be used to find out at any moment the incoming flows and reservoir volumes from the readings of the reservoir levels will be included in detailed design phase in chapter C.4 of this report, together with all the rating curves of each hydraulic device.

The most important hydraulic features, such as spillway, middle level outlet, ecological release valves, etc. of Batoka HEP are described in the Feasibility Design, to which reference is made.

Each of the two power plants is provided with the following generating facilities and equipment:

- No.6 Francis turbines with a rated output of 200 MW

- No.6 Main Inlet Valve, Butterfly type
- No. 6 synchronous generators
- No. 6 main step-up transformers

C.3.3 NORMAL OPERATIONS DESCRIPTION

The operating conditions of the power plant can be assumed to be "normal" whenever the plant is operated:

- within the equipment capabilities in respect to their nominal hydraulic, mechanical, electrical conditions

- with all the safety measures fulfilled by the Operator, according to
    - the O&M manuals of each single part/equipment/system of the power plant provided by the original manufacturers
    - the emergency preparedness plan specifically prepared for Batoka project

- without any outage or out of service of those facilities and systems which grant the safe operation of the equipment, such as
    - fire-fighting system
    - water drainage system of the Dam and Power House
    - Dam monitoring devices
    - Dam pumping station
    - SCADA system, HVAC system for ventilation and air conditioning, etc.

The "emergency" condition operation refers to a status of the plant wherein the occurrence of some events could affect the overall safe operation of the plant. For example, the prolonged outage of the generating units compels the running of the back-up supply (i.e. Emergency Diesel set and DC batteries system) in order to guarantee the availability of safety devices, such as the Dam Pumping system, the fire-fighting and HVAC systems, etc. The configuration of the power supply in such an event is given in the next paragraphs.

SURGE SHAFTS

Four (4) surge shafts, one per each waterway have been introduced into the system with the aim of reducing the amplitude of pressure fluctuations by reflecting the incoming pressure waves and providing stability to the system with respect to the small load variations.
The shaft diameter, the maximum and minimum water level in the surge shaft and the proper throttle orifice size were defined according to the results of the hydraulic calculations at feasibility design stage.

In the development of the detailed design further hydraulic analyses are assumed will be conducted, adopting the most severe combinations of turbines opening and closure maneuvers, with some freeboards considered. Whenever, in order to improve the stability in isolated operation condition of only one group of units (one

power waterway) a control signal device (Level Bias) will be introduced in the turbine governing system, the functioning, setting and operation requirements of this device shall be inserted in this report.

C.3.4 EXCEPTIONAL OPERATIONS DESCRIPTION

In principle, and considering the limited excursion of levels available at Batoka project, the power plants are not envisaged to work outside of the range of reservoir water levels indicated for normal operation case.

LARGE TRANSIENTS

The occurrence of the so called "large transients" are to be considered within the "exceptional" operating conditions of the power plant.
On each power waterway a throttled type surge shaft is adopted in order to improve the efficiency in damping oscillation due to load rejection or load acceptance. A throttling orifice, placed in the T junction between the final section of the power tunnel and the shaft itself, can increase the damping effect of friction losses.
In the development of the detailed design it is assumed that specific simulations will be carried out, considering the inputs of wicket gate closure time provided by the turbine supplier and different most critical scenarios of opening/closure of turbines.
Any prescription of possible requested time before reloading each unit (that in principle can be reloaded as quick as possible, without any time restrictions) that might come out from the detailed design hydraulic analysis shall be inserted in this document.

C.3.5 POWER SUPPLY FOR BATOKA PLANT OPERATION

This paragraph depicts the power supply sources available for the proper and safe functioning of the Batoka Plant, fed by main and backup power systems.

INTRODUCTION

The auxiliary power system in BATOKA HPP is conceived in such a manner to grant the supply in either "normal" and "emergency" operating conditions, whereas these conditions are defined in terms of (n-1) contingency. Whenever the outage of the "main" feeder occurs, the system will be able to automatically switch over the back-up feeder maintaining the supplied loads in service.
The "core" of the distribution system consists of a medium voltage (MV) bus, fed from different sources, namely:

- some of the turbine-generator units of each power house, via distribution transformers of adequate rated capacity
- the Diesel Emergency set installed in each Power House
- the unit auxiliary transformers in each power house, feeding their own auxiliary equipment

- the Dam auxiliary board, through a MV line, fed by the MV distribution board of the each Switchyard via MV overhead lines;
- the Diesel Generating Set installed at the dam

Each MV bus is configured in double-busbars with a coupler breaker for automatic transfer switching.

For the purpose of this document, some critical loads have been identified for the safety of the plant operation, i.e.:

- the Dam Pumping Station (DPS): this station provides drainage and pumping service for water collected by the drainage galleries of the dam
- the Dam Auxiliary System (DAS), which feeds the equipment relevant to the hydraulic structures equipment (service gate, hoists, etc.), emergency lighting, etc.
- the Power House general auxiliary system, which feeds the main equipment and device pertinent to unit auxiliary system, SCADA and communication system, HVAC system, emergency lighting, firefighting system, dewatering system, DC supply

The above identified loads will be fed in any operating status of the power plant (with/o the turbine-generators running). At the DAS, there is the provision of a spare bay for an additional Diesel set.

EMERGENCY OPERATION OF THE DPS AND DAS

In normal service, the two switchgears (i.e. the Dam Pumping Station and the Dam Auxiliary Board) are foreseen to be fed from the Power House through MV lines. In this case, dam and switchyard auxiliary loads are fully fed, and power flows from PH to DPS and DAS.

The emergency condition is intended to be a condition where both PH generating units and relevant distribution system are not available. The control system of the power plant, under recognized operating conditions, will automatically put the Diesel Generator Set, located at the Dam Pumping Station, in operation. In doing so, the control system will also provide to shed the non-essential loads, to allow energisation of the essential (and, among these, critical) loads.

Therefore, the LV switchgear of the Dam Pumping Station can be fed by the Diesel Emergency set and energise the DPS and the DAS. The control system (or the operator) shall then close the breakers corresponding to the loads at both switchgears (DPS and DAS). Power flows from DPS to DAS and SWY.

C.4 PLANT HYDRAULIC CONTROL DEVICES OPERATION

C.4.1 GENERAL

Description of operation for opening and closure manoeuvres for any hydraulic control device at Batoka (valves, gates, bulkheads, stoplogs, and relevant cranes and lifting devices) will be provided in the frame of the detailed design, and shall be then duly included in this report.
In this chapter the guidelines and basic principles assumed for the operation of the hydraulic devices are given.
It is recommended that specific operating procedures are implemented by the PMS, on the basis of the instructions provided in the present chapter, taking into account also the information and inputs of the O&M detailed manuals and drawings made available by the suppliers, to which reference shall be always made. Such procedures shall include at least the following scenarios:

- Hydraulic control devices operation under main power supply - remote and local control;
- Hydraulic control devices operation under back-up diesel power supply - remote and local control;
- Operation of back-up diesel generator and switch-over of power supply.
- Special conditions, if any, for first impounding.

It is worthwhile to recall that any maintenance or repair operation on gates or stoplogs shall be programmed and executed by qualified competent staff, possibly during dry season, avoiding as much as possible potential concomitance with floods events.

The calculation, verification, calibration etc. of the rating curves pertinent to the various discharge and control structures of Batoka project are described in the hydraulics analysis report of the Feasibility Design, to which reference is made.
The rating curves will be refined during the detailed design and construction phases, and for ease of reference and use they will be gathered here, including:

1. RIVER rating curve (at Power Houses location)
2. RESERVOIR rating curve
3. ECOLOGICAL DISCHARGE VALVES rating curve
4. MIDDLE LEVEL OUTLETS rating curve
5. SPILLWAY rating curves
6. POWER TUNNELS rating curves

C.4.2 INSTRUCTIONS FOR ECOLOGICAL FLOW RELEASE

The requirements for environmental discharge will be provided in the ESIA study.

The minimum discharge defined by the above-mentioned study, according to the water levels available in the reservoir within the minimum and maximum operating levels and to the Plant Operator choice, will be released by means of:

- the Power House units in operation (if at least one is maintained in operation 24 hours per day),
- the Ecological Discharge Device.
- the Spillway,
- the Middle Outlets.

Until the minimum operating level in the reservoir is not reached, the flood release shall be foreseen to be discharged:

- through the River Diversion scheme, until possible;
- through the Middle Outlets, as soon as possible compatibly with the Dam construction and reservoir water levels;
- through the Ecological discharge valves for reservoir levels above minimum operating level.

C.4.3 MIDDLE OUTLET UPSTREAM GATE OPERATION

GENERAL

Two Middle Outlet upstream gates are foreseen, one for each Middle Outlet.

The purpose of the upstream gate is to isolate the Middle Outlet waterways from the reservoir and allow inspection and maintenance activities on Middle Outlets structures and gates.

The upstream gate is operated the from the dam crest.

In normal conditions each upstream gate can remain closed or opened, upon the Plant Owner choice. The closed position is safer for the dam because avoids to maintain the water pressure inside the dam. It has the disadvantage that the gates remain for long period under water.

The layout and description of the upstream gate with relevant hoist and auxiliary equipment will be indicated here as developed in detailed design by the supplier.

OPERATION

The bulkhead will be closed whenever an inspection/maintenance/repair of the Middle Outlet steel lining and/or gates has to be carried out.

The gate shall be operated under balanced pressure conditions.

Before lifting the gate, the by-pass valve shall be opened in order to equalize the water pressure (time required to fill the whole waterway will be provided by the gate (and by-pass) supplier in detailed design).

When the water flows through the Middle Outlets, the upstream gate must be lifted as necessary (about 15 to 20m minimum above the bottom sill, to have the whole intake of the MLO fully opened (time required to lift the gate above the bottom sill, including the time to remove one rod, will be provided by the gate (and by-pass) supplier in detailed design).

Typically a locking device will allow the removal and the insertion of the lifting rods of the gate.

Only for its inspection, maintenance or repair purposes the upstream gate can be raised up to the dam crest.

Before closing the gate, the MLO downstream gates must be closed. The gate will be lowered by gravity, from the dam crest.

C.4.4 MIDDLE OUTLETS DOWNSTREAM GATES OPERATION

Each Middle Outlet device will be equipped with two downstream gates, named gate no. 1 upstream and gate no. 2 downstream, both at the outlet section:

- The gate no. 1 is an emergency gate that shall be closed only in extraordinary need of gate no. 2 maintenance, and it normally works fully opened. The gate will operate in balanced condition, but in case of need of sudden closure, the gate shall be able to work in unbalanced condition.
  This kind of exceptional closure shall be carried out only in case of real emergency, and after having made all attempt to release the gate no. 2, because such operation implies high risk of cavitation and dangerous pressure pulsation in the whole downstream portion of the Middle Outlet conduit.
  In case of necessity of closure in unbalanced condition, the operation shall be programmed to be executed minimizing its duration.
  In case of need of gate no.1 inspection or maintenance, the gate no. 2 shall be closed to empty the Middle Outlet conduit.
  All the operations gate no. 1 and gate no. 2 are conducted by means of the pertinent hydraulic units that are foreseen to be commanded directly locally and also connected to the Dam Control Building for remote control.

- The gate no. 2 shall work fully closed or fully opened. It is normally closed and it is opened and closed in unbalanced conditions, at any need of Middle Outlet use. This gate is placed d/s the gate no. 1 and shall have a circular cross section.
  In case of need of gate no. 2 inspection or maintenance, the gate no. 1, that is upstream, shall be closed to empty the Middle Outlet conduit between the two gates.

From an operational point of view, the Middle Outlets can operate within their operating levels, in pressurized conditions, with jet falling in the Plunge Pool.

For lower levels MLO flows can pass from pressurized conditions to free flow conditions, with progressive reduction of water jets and, in the transient conditions, with possible entrainment of air in the conduit. The possible temporary or exceptional use of MLO out of the design range of operation shall be object of investigation at detailed design phase (possibly through physical model tests).

It is therefore recommended to use the Middle Outlets in pressurized conditions, keeping them closed by means of the downstream gates for reservoir water levels below their minimum pressurized operating level. Below such level the use of Middle Outlets is not recommended, and in any case shall be limited in time to minimize risks of possible damages to the dam downstream face and Middle Outlet conduit.

It is recalled again that:

- The upstream gate shall be opened always in balanced conditions.
- The downstream gate no. 1 can work only in fully closed or fully opened conditions
- The downstream gate no. 2 can work only in fully closed or fully opened conditions. It shall usually work in balanced conditions, and only in emergency case it can work in unbalanced conditions.

The closing speed for both gates will be defined by the supplier, for any normal or exceptional condition.

C.4.5 RESERVOIR DRAWDOWN

The reservoir drawdown is foreseen in two cases:

- During the dry season to allow the rafting activities on the Zambesi river upstream the dam
- In case of necessity to inspect:
    - lower portions of dam upstream face or
    - the first portion of Power Tunnels upstream of the gate shafts

The reservoir drawdown operations shall be carried out according to specific procedures that will be defined during the frame of the detailed design of the hydraulic structures.

The requirements for lowering of the reservoir during dry season for rafting activities will be provided in the ESIA study.

The reservoir will be lowered, according to the water levels available in the reservoir within the minimum and maximum operating levels and to the Plant Operator choice, by means of:

- the Power House units in operation (if at least one is maintained in operation 24 hours per day),
- the Ecological Discharge Device.
- the Spillway,
- the Middle Outlets.

Same criteria for lowering the reservoir can be applied in case of necessity of inspection of lower portions of dam upstream face or the first portion of Power Tunnels upstream of the gate shafts.

C.4.6 SPILLWAY GATES OPERATION

The Spillway gates can operate from el. 738.6 to el. 762 m a.s.l.

The operation of the Spillway shall be programmed by the Plant Owner in function of:

- Power Production and energy market requirements;
- flood release requirements;
- water availability, monitored through the river and reservoir water levels.

OPERATION

The sequence and degree of aperture that shall be adopted for gates opening and closure will be defined in detailed design phase, considering the Spillway physical hydraulic model tests that are assumed will be conducted, and taking into account that for ordinary flows the use of the spillway can be limited to some gates only, while for higher flows in principle all gates will be required to function simultaneously.

Analogue procedure will be reversely defined and applied for gate closure when the incoming flow decreases. The automatism for this procedure will be implemented by the gate supplier.

The control of the gates will be made locally (on the control unit foreseen near each gate) and remotely from the Dam Control building.

In case of one gate is blocked, or for any reason out of operation, when the Spillway shall be used, depending on which gate is out of operation the sequence defined for the normal operation criteria shall be rearranged consequently, with the aim to obtain the configuration of jets discharging in the pool as close as possible to the one originally envisaged.

The intervention of release, inspection, maintenance or reparation on a blocked gate shall be conducted by isolating the bay upstream side by means of the stoplogs, which use and operation is described in the next paragraph.

C.4.7 SPILLWAY STOPLOGS OPERATION

GENERAL

The purpose of the Spillway stoplogs is to allow the inspection of the Spillway structure and radial gates.
The stoplogs set will be made of multiple interchangeable elements. The elements will be designed to be lowered or raised in the slots in a vertical position after the release of the lifting beam by suitable grappling beam, of a self-latching and unlatching type.

OPERATION

The Spillway stoplogs will be operated by the crane, running along the dam crest structure.

The lifting and lowering of the stoplog elements is done only under balanced head, under zero flow. This means that, before opening or lowering of the stoplog, the radial gate must be completely closed.

The Spillway Stoplog INSERTION PROCEDURE is articulated as follows:

1. Ensure that the Spillway radial gate is closed
2. Engage the lifting beam with the crane
3. By mean of the crane, lower the lifting beam and engage the stoplog element which lies in the stored position.
4. Lift the stoplog element and check for correct stoplog engagement
5. Lower the stoplog element in the sluice till it reaches the sill
6. When the stoplog rests on the sill beam (the lifting beam automatically disengages)
7. Lift the beam and repeat steps from (3) - (7) with the others stoplog elements

The Spillway Stoplog REMOVAL PROCEDURE is articulated as follows:

- Ensure that the Spillway radial gate is closed
- Engage the lifting beam with the crane hook
- Lower the lifting beam in the sluice to engage the upper stoplog element
- Lift the upper stoplog element under unbalanced conditions for 100mm approximately
- Wait until balanced pressure condition is established.
- Lift the stoplog until the operation level and store it in the proper structure (the lifting beam automatically disengages).
- Lift the beam and repeat steps from (2) - (6) with the others stoplog elements

The details and procedures for operating lifting beam and the movement of the hook during insertion and removal of the stoplog will be provided by the supplier in detailed design phase, and included in the final version of the operation manual, as well as the stocking area and stoplogs handling and stocking details and procedures.

C.4.8 INTAKE BULKHEAD GATES OPERATION

GENERAL

The bulkhead gates will be installed in the intake tunnels upstream from the intake wheel gates. The main purpose of these gates is closure of the waterways to allow maintenance and inspection of:

1. the intake wheel gates
2. the power tunnels
3. the penstocks
4. the Main Inlet Valves.

OPERATION

Bulkhead gate is foreseen to be of slide type operated by stationary cable hoist, placed at the shaft top. Gate and hoist are assumed to be connected directly.

The bulkhead gates shall normally be kept in open position. For this reason locking devices are foreseen at the upper floor of the shaft.

Closing of the bulkhead gates shall be done under balanced pressure condition. For this reason, before the bulkhead are closed, the closure of the fixed wheel gates and/or main inlet valves must to be assured.
Opening of the bulkhead gate shall be done under balanced pressure condition.
In order to equalize the water pressure on both side, one by-pass valve is foreseen to be incorporated into the upper gate leaf. Before the bulkhead is lifted, the by-pass valve must be raised as necessary to equalize the water pressure filling the gap between the bulkhead and wheel gate. The gate normal opening speed will be set by the gate supplier, it is however recommended to maintain as far as possible low velocities (in the range of 0.5 m/min).

C.4.9 INTAKE WHEEL GATES OPERATION

GENERAL

Two gates of wheel type are foreseen to be placed in each power waterway of the power plant. These gates are installed in the intake tunnel downstream from the bulkhead gates.
The main purpose of these gates is the closure of the waterways to allow:

1. maintenance and inspection of:
    - the power tunnels
    - the penstocks
    - the Main Inlet Valves
5. emergency closure in case units are under load rejection and wicket gates and main inlet valves fail to close.

OPERATION

Each wheel gate will be operated by an oil pressure servomotor, mounted at the top of the shaft. Connection between the gate and piston rod is foreseen to be made by means of flexible connected lifting rods.
The wheel gates shall be normally kept in open position, by means of locking devices on gate shaft roof.
The wheel gate shall be used as an emergency gate. Closing of the wheel gate is done under unbalanced pressure condition. The wheel gate is foreseen to be capable to close under the full flow in case of emergency. For this reason, the wheel gate will be designed to be able to close by its own weight (including lifting rods) and by water column on the top of it, under any flow condition. While closing, the servomotor can act as a brake.

The gate is opened by oil pressure of the servomotor. Opening of the bulkhead gate must be done under balanced pressure condition. In order to equalize the water pressure on both side, one by-pass valve is foreseen to be incorporated into the upper gate leaf. Before the wheel gate is lifted, the by-pass valve must be raised to equalize the water pressure. The gate normal opening speed of the gate will be set by the supplier, it is however recommended to maintain in any case low velocities (in the range of 0.1 m/min).

C.4.10 POWER WATERWAYS EMPTYING AND FILLING OPERATION

The EMPTYING PROCEDURE of one waterway shall be defined in the detailed design, identifying what are the steps of closing and opening of each hydraulic or electric device, and relevant timing or possible constraints, taking into account the rate of velocity for the opening and closure procedure of the gates, and details about gates functioning as will be provided in relevant EM-HSS equipment operation and maintenance manuals.

In case power waterways empting is necessary, a large amount of water (until the river water level is reached) can be discharged in any condition through the Power House.
Thereafter penstocks emptying pipes, foreseen for each of the manifold branches, provided with gates valves manually operated, allow to discharge only the last volume of water inside penstocks, below the river level. Water inside draft tube, downstream of main turbine valves, will be drained by the dedicated pipes.

The details and the hydraulic calculations of the penstocks emptying procedures and relevant time requirements will be developed at detailed design stage.

The FILLING PROCEDURE of one Power waterway after an emptying process shall be defined in the detailed design, identifying what are the steps of closing and opening of each hydraulic or electric device, taking into account that the selection of the water pressure rising rate in the Power Waterways system shall be fixed considering any requirement of the steel penstocks prescribed by the Plant Contractor.

C.4.11 ACCESS TO EMPTY POWER WATERWAYS FOR INSPECTION & MAINTENANCE PURPOSES

The inspection of the horizontal part of tunnels downstream of the gate shafts can be made, once empty, getting access from the watertight doors foreseen to be available on the external side of each tunnel, upstream of the steel lined portion.

Inspection of steel lining of penstocks e Manifolds, as well as the surge shaft can be done accessing through:

1. Tunnels Watertight doors above mentioned, for their horizontal stretch (the surge shaft crest can be reached by stairs from external side).

2. Manholes on the upper bend of the steel lining penstocks (a chamber with hoist is foreseen to be available for each penstock, accessible from the dedicated upper access tunnel serving also as drainage tunnel).
3. Manholes upstream of the Main Inlet Valve, accessible from the Power House.

Access must be restricted to qualified people appointed and authorized by the PMS. A strict control shall be done in such exceptional maintenance operations that no free access is given to the empty tunnel.
Before entering the empty tunnel it must be verified that (if water level of the reservoir is not maintained below the tunnel invert level) the upstream gates are closed and don't leak, and that the conditions of water presence, temperature and humidity conditions inside the tunnel are compatible with the safety of the workers.
It shall be implemented a mobile communication system between people entering in the tunnel and people governing and monitoring the gates and the reservoir.

The exceptional inspection of the portal upstream of the gate shaft can be made only if the reservoir is lowered and maintained below the invert level of the tunnel.

Unless it is possible and convenient to lower the reservoir level (by means of the MLO) below the invert level of the tunnels, and to maintain such condition for the whole period needed for maintenance, the inspection and maintenance operations in the tunnel shall be conducted having closed the gate shaft gates, and being verified the certainty of such closure before entering in the tunnel.
Detailed procedures will be developed in detailed design phase and inserted in the final operation manual. As a general prescription, it is here recalled that the inspection of the tunnel should be programmed preferably dewatering one tunnel at the time, or in any case two at the time but not the two tunnels of the two plant simultaneously (unless strictly necessary), and in any case this exceptional operation shall be programmed in advance to allow to manage the consequences of a long period of plant(s) stop, and any period of water discharge through the Spillway/Middle Outlet.

The inspection of the main tunnel (and the eventual repair or maintenance operations required) shall be done only by skilled personnel, instructed and authorized by the Plant Owner for the operation required, with the assistance of a design specialist.

C.4.12 MAIN INLET VALVES OPERATION

GENERAL

Ten main inlet valves (MIV) are foreseen to be placed upstream each hydraulic unit.
The main purposes of these main inlet valves are the following:

- Emergency closure, with maximum flow under all extreme conditions, including the transient forces caused by water-hammer. Such condition occurs with a turbine accident as well as the guide vane

fails to close. The prompt closure of the valve assures safety of the turbine unit and avoid the turbine over-speed.

- Closure for Normal shut down of the unit, under balanced pressure conditions with the wicket gates completely closed. The closure of the valve prevents the leakage of the wicket gates for a longtime and reduces the water loss.
- Closure for maintenance of the unit, under balanced pressure conditions with the wicket gates completely closed.

DESIGN

The MIV is made by two major parts:

2. Valve body that includes the driving mechanism, valve body, valve disc, valve shaft, shaft seal components, automatic locking oil cylinder, hydraulic system and so on.
3. Auxiliary components, that includes bypass valve, bypass pipe, inlet pipe, outlet pipe and expansion joint, the hydraulic system and so on.

The MIV has two seals: the main seal (also named work seal) and maintenance seal. When the main seal is working normally, the maintenance seal is released. If the valves need maintenance, the maintenance seal is used.

OPERATION

The MIV opening is driven by oil operated servomotor; during the valve opening, the counterweight is lifted up to provide energy for closing. In fact the MIV closure is done by gravity, with counterweight.

Normal opening and closing will be under balanced pressure, with the by-pass valve opened and wicket-gates in closed position: suitable interlocks prevent the valve opening when pressures have not the same values.

When the valve is fully opened, the pressure of hydraulic system is maintained automatically. The hydraulic system will keep the oil pressure automatically when the valve is full open. The pump will stop when the oil pressure reaches the upper limit whereas the oil pump will start when the oil pressure drops below the lower limit. This function is available when the full open signal is sent out.

The hydraulic system has a redundancy closing function. This function allows the oil pressure to enter into the piston rod chamber with the purpose to help the valve closure. This function is not activated for normal or emergency closures. The redundancy function shall be activated when the friction force will increase, typically after many years of operation.

C.4.13 DRAFT TUBE GATES OPERATION

GENERAL

What follows refer to one power house and it is applicable of both Power Houses of Batoka Project.
Each powerhouse has six hydraulic turbines. Downstream of each turbine there is a draft tube that, after the elbow, is divided in two parts by a pier. Each draft tube has two sliding draft tube gates, one for each sluice of the draft tube.

The draft tube gates will serve for maintenance purposes only, in fact their main purpose is to isolate the turbine and draft tube from the tailrace. During normal operation these gates will be kept in the fully open position by a self-engaging and disengaging safety latch conveniently located either on the servomotor top cover or on its bottom pedestal.

The draft tube gates will be closed to dewater and to carry out maintenance in the turbines and draft tubes.

OPERATION

The gates will be raised and lowered by an oil operated servomotors. The draft tube gates are usually operated under balanced pressure on upstream and downstream sides. To this purpose each gate will have its own by pass system provided to allow the filling of the draft tube (with times that will be indicated by the gate supplier in detailed design phase). When the pressure on both sides of the gate is balanced, a pressure switch gives the consent for opening.

The control of operations of the draft tube gates can be done from local control panels and from Power House control building.
In case of maintenance the gate is lifted by a monorail crane.

C.4.14 DRAFT TUBE STOPLOGS OPERATION

GENERAL

For each draft tube two draft tube sliding gates are foreseen (see previous paragraph). Downstream these gates there is a slot for the draft tube stoplogs. There are typically two sets of draft tube stoplogs that will allow maintenance for two units at the same time.
The purpose of the draft tube stoplogs is to allow the inspection and maintenance of the draft tube sliding gates and draft tubes.

Each stoplog set of the draft tube is divided in elements. The bottom and top elements are typically not interchangeable because the top element is designed with a top seal against the sluice upper face. The elements are designed to be lowered or raised by suitable grappling beam, of a self-latching and unlatching type, they are operated by the draft tube crane from upper floor of PH.

In normal condition, when the generating units are working, each element of the stoplog is stocked in the relevant storage area. Steel grid covers are foreseen for closure the stoplogs slots at power house upper floor. During maintenance of powerhouse electro-mechanical equipment, the stoplogs are placed on both sluices of any draft tube, to close the water flow from the tailrace side.

OPERATION

The draft tube stoplogs will be operated by the relevant crane, running along the power house deck structure at upper floor. The storage area for the draft tube stoplogs is in the close proximity of the sluices and accessible by the hoisting facility.

The lifting and lowering of the stoplog elements will be done only under balanced head, under zero flow. This means that before opening or lowering of the stoplog it has to be sure that units are stopped and, preferably, draft tube sliding gates are closed.

The Draft tube Stoplog INSERTION PROCEDURE is articulated as follows:

- Ensure that the unit is stopped
- Ensure that the draft tube sliding gates are closed
- Engage the lifting beam with the draft tube crane
- By mean of the draft tube crane lower the lifting beam and engage the stoplog element which lies in the stored position.
- Lift the stoplog element and check for correct stoplog engagement
- Lower the stoplog element in the sluice till it reaches the sill: during this operation make sure that the beam guide shoes are properly engaged with the rails fixed in the concrete.
- When the stoplog rests on the sill beam, the lifting beam automatically disengages
- Lift the beam and repeat steps from (4) - (8) with the others stoplog elements

The Draft tube Stoplog REMOVAL PROCEDURE is articulated as follows:

- Engage the lifting beam with the crane hook.
- Lower the lifting beam in the sluice to engage the upper stoplog element: during this operation make sure that the beam guide shoes are properly engaged with the lateral fixed in the concrete.
- Lift the upper stoplog element under unbalanced conditions for 100mm approximately
- Wait until balanced pressure condition is established.
- Lift the stoplog until the operation level and store it in the proper structure: the lifting beam automatically diseganges.
- Lift the beam and repeat steps from (2) – (6) with the others stoplog elements

The description of the lifting beam and the movement of the hook during insertion and removal of the stoplog will be provided by the supplier in detailed design.

C.5 OTHER PRESCRIPTIONS FOR CIVIL WORKS OPERATION

C.5.1 GENERAL

The inspection of the works and the areas downstream of the dam shall be frequently conducted with the aim to find and measures any signal of potential leakage or static problems to the structures connected with the operation of the plant.
The monitoring of all the instrumentation foreseen shall be regularly conducted, following all the prescriptions that are provide in part B of this Dam Safety Plan (to which reference is made for all instrumentation monitoring activities), and that shall be updated and detailed during the development of detailed design and construction phases.

In addition, particular attention shall be paid to the hydraulic steel structures when they enter into operation, and to the Dam also in the upstream zone (as far as possible to be inspected from upstream and from the dam galleries).

All the above mentioned maintenance requirements are described in the Part D of the Dam Safety Plan (to which reference is made), and that shall be updated and detailed during the development of detailed design and construction phases.

Other specific prescriptions for the use or operation of the civil works will be typically provided during the detailed design and construction phases, to be included here.

C.5.2 RESTRICTED AREAS

In general is mandatory to restrict the access to the Plant to the authorized people only.

A tentative preliminary list of specific locations that require to be locked for safety reasons is provided hereinafter. The PMS shall maintain in good status and available for inspection of authorized people the accesses, the gates or doors or fences and relevant locking devices.
- Dam galleries
- Dam cable shaft
- Dam control building
- Dam crest and abutment yards
- Dam spillway gates control room
- Dam middle outlet gates control room
- Dam pumping system control building
- Dam pumping system pits
- Power Tunnels access tunnels to watertight doors and manholes

- Power Houses
- Power Houses diesel generator buildings
- Switchyards
- Switchyards control building
- Gate shafts and relevant yards
- Surge shafts and relevant yards
- Reservoir slopes around the Power Tunnel intakes.

It is to underline that the specific transboundary nature of Batoka Project implies the possible necessity of having border control posts on dam abutments, and specific regulations and rules for the access to the dam (for personnel working at Batoka Plant) and for persons that want to cross the border, to be defined and detailed by the Zambian and Zimbabwean Governments and implemented by the relevant Authorities.

The access inside the Power House, Switchyard and Dam control building and to the zones where there are EM equipment in tension (like the transformers) shall be restricted to the only persons authorized by the PMS, that shall be aware of the functioning of the equipment and control system.

At site, in visible places, panels shall be provided to inform clearly about the above prescriptions and limitations.

The control room of the Power Houses shall be manned, to allow adequate control of the Dam and Power Houses behavior in real time.

All the EM and HSS equipment present in the Power Houses shall be object of operation and maintenance activities as prescribed in the relevant manuals of the supplier.

The ordinary maintenance and cleaning of civil works and architectural components is required for the functionality and durability of the Dam, Shafts, Power House, Switchyard, as well as for all the other civil work structures, and it shall be therefore considered and planned as a part of the plant operation program, as described in part D of this report.

C.5.3 LOADS AND ACCESS LIMITATIONS

There are typically some restrictions for the loads allowed on the Power House erection bays and on dam crest loading areas.
Some zone are not designed for heavy loads and in others there are loads limitations and special prescriptions (for instance in power house erection bay for the exceptional case of two units erected simultaneously), or specific areas for stoplogs or other heavy equipment stock.
Also there will be restrictions for the access to vehicles on some areas: some zones will be not designed for vehicular loads and in others there will be loads limitations.

The stoplogs for the Power House tailraces shall be stocked in the specific areas identified by the detailed design, and the same applies for the stoplogs for the Dam spillway.
The prescriptions will be provided in detailed design and included in this manual with appropriate plan drawings showing areas and limits of loads, as required.

At site, in visible places, panels shall be provided to inform clearly about the above prescriptions and limitations.

C.5.4 TRANSFORMER OIL WATER RECOLLECTION SYSTEM OPERATION

Here below there is a brief typical description of the Power House transformers oil recollection and disposal system that will be defined and sized in detailed design.
Below each transformer a small basin partially filled with gravel will recollect in case of fire event the firefighting system waters and any oil leakage.
The basins will be connected to a steel pipe that conveys all the oil and water in a dedicated tank, buried at the extremity of the Power House.
In the tank the oil and the water are separated by gravity. Oil is foreseen to be recollected and disposed appropriately, water to be discharged in the river.

The final recollection tank will have a volume capable to contain the maximum possible amount of fire extinguishing waters, plus the transformer oil.
Two internal walls are also foreseen to facilitate the oil water separation, in order to avoid in any case that the oil is discharged directly in to the river.
There is a water bottom scour pipe on the bottom of the final recollection tank, for water discharge, discharging directly into the river. A portable pump is foreseen for oil disposal.

Two cases will be considered:

Case 1) Transformers oil leakage:
Should any one of the transformers have some oil leakage, the basin beneath transformers will collect the oil and dispose it in the final recollection tank. Such oil will be not pumped out by the water pump, since the corresponding volume is considered in the dead capacity of the final recollection tank.
In case of important leakages oil must be recollected from the final recollection tanks by means of portable pump, as described in the next case.

Case 2) Transformers fire:
Should any of the transformers be damaged by fire, the tanks beneath transformers will collect the oil that is discharged, together with the water used for the fire extinguishing, and dispose the oil and the water in the final recollection tank.

After some time, once oil and water are separated, the floating oil stratum in the final recollection tank will be removed by use of a portable pump on hand at the PH Mechanical Store.
Such pump is utilised by the appointed personnel who will manually insert the pump into the tank and position it to pump out only the oil which will be disposed of in barrels.
After this operation the water still present in the tank is foreseen to be discharged by manually opening the valve of the bottom outlet pipe.

C.5.5 PLUNGE POOL AND TAILRACE CLEANING

The tailrace hole (in front of the exit of the Power Houses draft tubes) shall be maintained free of stones and debris, to avoid clogging of the waterways of turbine outlets.
To this purpose periodic check of the tailrace hole shall be conducted (see Part D of the report), and in case it is found material accumulated in the bottom of the tailrace hole, at the exit of the draft tubes, if such deposed material is above the invert level of the draft tube canal, it shall be removed.
Material can be removed by using a barge or by similar equipment. Such operation shall be done with the correspondent unit not in operation.

Debris, loose rock elements or boulders accumulated in the plunge pool bottom do not constitute a problem, until they remain inside the pool. If, for instance after a flood event or a severe condition of use of the spillway, such material is transported by the currents out of the pool, if it falls inside the tailrace the above recommendations of tailrace cleaning apply.

After a flood event or a severe or prolonged condition of use of the spillway, it is recommended to check the status of the dam downstream toe. See Part D of the Dam Safety Plan.

C.5.6 USE of RESERVOIR FLOATING BARRIER

Floating barrier is typically conceived as instrument to protect from entrance of debris and trunks the following hydraulic works:

1. Middle Outlet,
2. Power Tunnel Intakes
3. Spillway.

Its use is not strictly necessary but recommended for the first impounding (especially to protect MLO and Power Waterways intakes) and also for the operation period, especially for the protection of the spillway.
Also for the emergency case (exceptional events) of reservoir drawdown, it can be used for the subsequent re-impounding.

If a barrier is adopted, maintenance and periodic cleaning of the material accumulated against the barrier shall be executed by the PMS, accessing to it by means of access roads surrounding the reservoir in the zone near the dam, or by barge, with frequency depending on the ground condition (floods, accumulation of debris), as set in Part D of Dam Safety Plan.

More details in this regard are assumed will come from the detailed design and construction phases.

C.5.7 ACCESS TO DAM AND RELEVANT GALLERIES

The access to the galleries inside the dam body will be permitted by the access galleries foreseen on both banks. From the dam crest (near the abutments) there will be the entrance to the upper gallery, from which also the cable shaft inside the dam and the pendula shafts will start.

It is strongly recommended to do not access with any vehicles the dam downstream face benches, with the exception of the wider ones designed as access roads for the galleries or HSS devices.

In any case access inside the galleries with vehicles shall not be allowed, because the galleries are usually not ventilated to cope with vehicles gas emissions.

All benches of the dam downstream face are typically not protected with any guard rails or fences, therefore if it is needed to pass with any vehicle on the dam, also along the access roads, in case of need for maintenance or repair works, a temporary fence shall be built where it is intended to get access, adequately sized to prevent any accident or falling down.

The gates giving access to the dam galleries shall be kept closed, for safety reasons, and the entrance shall be allowed to authorized persons only.

The lowest galleries usually have no direct access at their level. They are accessible only from the dam downstream face by shaft, typically equipped with stairs, for pedestrian access, and with a hoist, to lower equipment for maintenance, drilling or grouting operations. Here also no permanent ventilation system is usually present; it shall be provided on temporary basis in case equipment are introduced and prolonged works shall be carried out inside these galleries.
The same shafts serve the pumps pits on their bottom.
A cable shaft, equipped with a lift (for tools, not for persons) and staircase, puts in communication the galleries at different elevations. In case of emergency it can be used to enter or escape from another gallery.

In occasion of the periodic monitoring of the drainage system inside the galleries, it is recommended to carry out a cleaning of any obstruction, debris or other element that can be an obstacle to the discharge of the water.

C.6 ORGANIZATION, TRAINING AND FACILITIES FOR PLANT OPERATION

C.6.1 ORGANIZATION OF THE PLANT MANAGEMENT STRUCTURE

Details of the operational structure are defined by the Plant Owner and Plant Management Structure (PMS).

The PMS will be responsible for the maintenance of the dam, including the dam safety aspects. PMS will also be responsible for the operation of the dam and coordination of operation of the power houses, and in particular the operation of the hydraulic control devices.

They will prepare the reservoir operating rules for the power supply, reservoir and river base flow demand rules, taking into account the needs of an integrated operation of both Batoka Power Houses as well as the Batoka-Kariba cascade.

They will fill the formats provided at par. E.7.4 and they will use them for all needs.

Within this frame PMS will also develop daily operating arrangements for water releases from the reservoir.

The PMS defines how much staff shall be employed for the operation and maintenance of the plant. The following is a minimum anticipated requirement to carry out the basic dam maintenance and safety requirements for Batoka Plant:

- For operation of the flow control gates, valves and hydraulic devices, one operator (preferably two men for safety reasons) available when required.
- Monthly inspections and reports will require one man (preferably two men for safety reasons) to carry out the inspection and monitoring.
- It is recommended that the main valves and gates maintenance (both mechanical and electrical) are covered by a maintenance contract under PMS.

Additional people will be required as and when necessary to carry out larger maintenance work or specialist work.

C.6.2 OPERATORS TRAINING

The operators shall undergo a training programme covering the technical aspects of the dam and plant equipment and their operation. The programme shall cover both the features and performance of the equipment and their operation, to be illustrated both through off-site theoretical and on-site practical training sessions. These training sessions should be carried out by the manufacturers of the equipment or someone else with an intimate knowledge of the equipment and of the plant functioning.
Specific training to learn how to use, monitor and maintain the instrumentation installed at dam is required.

Operators must have a certificate of competence, which will be recorded in a training register.

Maintainers must have a certificate of competence, which will also be recorded in a training register.
Both the operators and the maintainers must attend training courses covering:

1. The dam safety in emergency situations, through which they should gain a comprehensive awareness and competence in emergency identification and emergency actions.
2. Personal and general safety in the workplace.

The above training should be fully documented in a training register.

C.6.3 EQUIPMENT AND FACILITY REQUIREMENTS FOR BATOKA PLANT

In addition to the equipment installed (gate hoists, controls and emergency diesel generator) in detailed design it will be identified if there will be other equipment needed for operation and maintenance of the plant, for instance items (like for instance mobile cranes, or barges, or special tools) that might be required for exceptional maintenance cases.

A full list of the spares provided by the manufacturers and suppliers shall be added to this O&M manual by the PMS for a prompt consultation in case of need.

The list shall be carried out after check of all the items present on site filling a format like the one provided in sake of reference in the next paragraph.

C.6.4 FORMAT FOR SPARE PARTS LIST

Add lines and duplicate as far as necessary.

BATOKA SITE SPARE PARTS LIST											
LOCATION		ITEM			AVAILABILITY						
area	room	Description	Quantities	Taken in date	By (insert name and signature)	Returned in date	By (insert name and signature)	Used or consumed in date	By (name and signature)	To be re-ordered. Ordered in date	By (name and signature)
PH	*Mechanical Store*	*Pumps for oil dewatering*									
Dam	*Control Buidling*	*Mechanical tools box*									

Table 9 - Table of Batoka Site spare parts list format

C.6.5 CONTACTS FOR ROUTINE OR ALERT PROCEDURE IMPLEMENTATION

The data gathered from the instrumentations, as described in the Parts B, D and E of the Dam Safety Plan, and other reports and data collected in ROUTINE or ALERT conditions (ref. Part B of Dam Safety Plan) shall be forwarded to the following contacts:

Plant Management Structure				
Name	Position	Telephone	E-mail	Mail address

Table 10 - Contacts for Routine or Alert procedure implementation

C.6.6 CONTACTS FOR EMERGENCY or ALARM CASES

In case of need to trigger an ALARM condition, or in case of need to implement dedicated procedures described in Part E of the Dam Safety Plan, the addresses reported here below shall be immediately contacted:

Plant Management Structure				
Name	Position	Telephone	E-mail	Mail address

Local/Governmental Authorities				
Name	Position	Telephone	E-mail	Mail address

Table 11 - Table of Contacts for emergency or alarm cases

PART D - MAINTENANCE PLAN (Preliminary Plan)

D.1 INTRODUCTION

D.1.1 CONTENT AND STRUCTURE OF THIS PART

This is the PART D "**MAINTENANCE PLAN"** of the Batoka Dam Safety Plan.
This part of the report, and its references, comprises the guidelines for the Maintenance Plan of the Batoka Plant, including the dam, power waterways, power house and other appurtenant structures.
It outlines the operation and maintenance activities and procedures relevant to the Batoka Dam and Hydro Power Plants, to be detailed and implemented by the Project Management Structure that will be appointed by the Owner(s) of the Plant.
Being the design at Feasibility stage, this document has a Preliminary nature, being necessary to be developed and detailed during project implementation. The final plan is due prior to 6 months before initial filling of the reservoir according to the development and detail of the design, and further on according to the construction of the plant itself.

The project has a transboundary nature, having the dam shared between Zambia and Zimbabwe.
The Zambesi River Authority (ZRA) is a transnational authority that deals with the Zambesi river, formed by the Council of Ministers of Zambia and Zimbabwe, the Board of Directors and the Executive Management, and it is assumed to be the organ charged to be responsible of the operation and maintenance of Batoka Dam. The two national Plants linked to the dam can have different structures of Ownership and Operation, and that when will be appointed (by ZRA or by Zambia and Zimbabwe governments), will take the responsibility of the operation and maintenance of the Plant(s).

According to the structures of Ownership and Operation that Zambian and Zimbabwean countries and ZRA will put in place, the responsibility of maintenance of the dam and of the two plants can be assigned to different PMSs, that, in the frame of development of the manuals and of relevant implementation rules and structures, will organize (dividing and coordinating as appropriate) the work to be done and the limits and coordination of responsibilities. At the present stage of the design, and considering the importance to have an overall view of the Batoka scheme operation and maintenance requirements, the document is developed as one manual for the whole scheme.

As far as the Batoka Plant MAINTENANCE instructions are concerned, it is firstly illustrated the procedure to be adopted for the inspection and maintenance of the Plant, and in the subsequent chapters procedures are provided, as far as civil works are concerned, dedicated to each single work constituting the plant.
It is assumed that Electromechanical and Hydraulic steel structures equipment and devices detailed Operation and Maintenance manuals will be provided by relevant suppliers before Plant Commissioning: such manuals will be gathered and make part of the detailed Operation and Maintenance manual of the Plant.

D.2 PROCESS OF INSPECTION AND MAINTENANCE

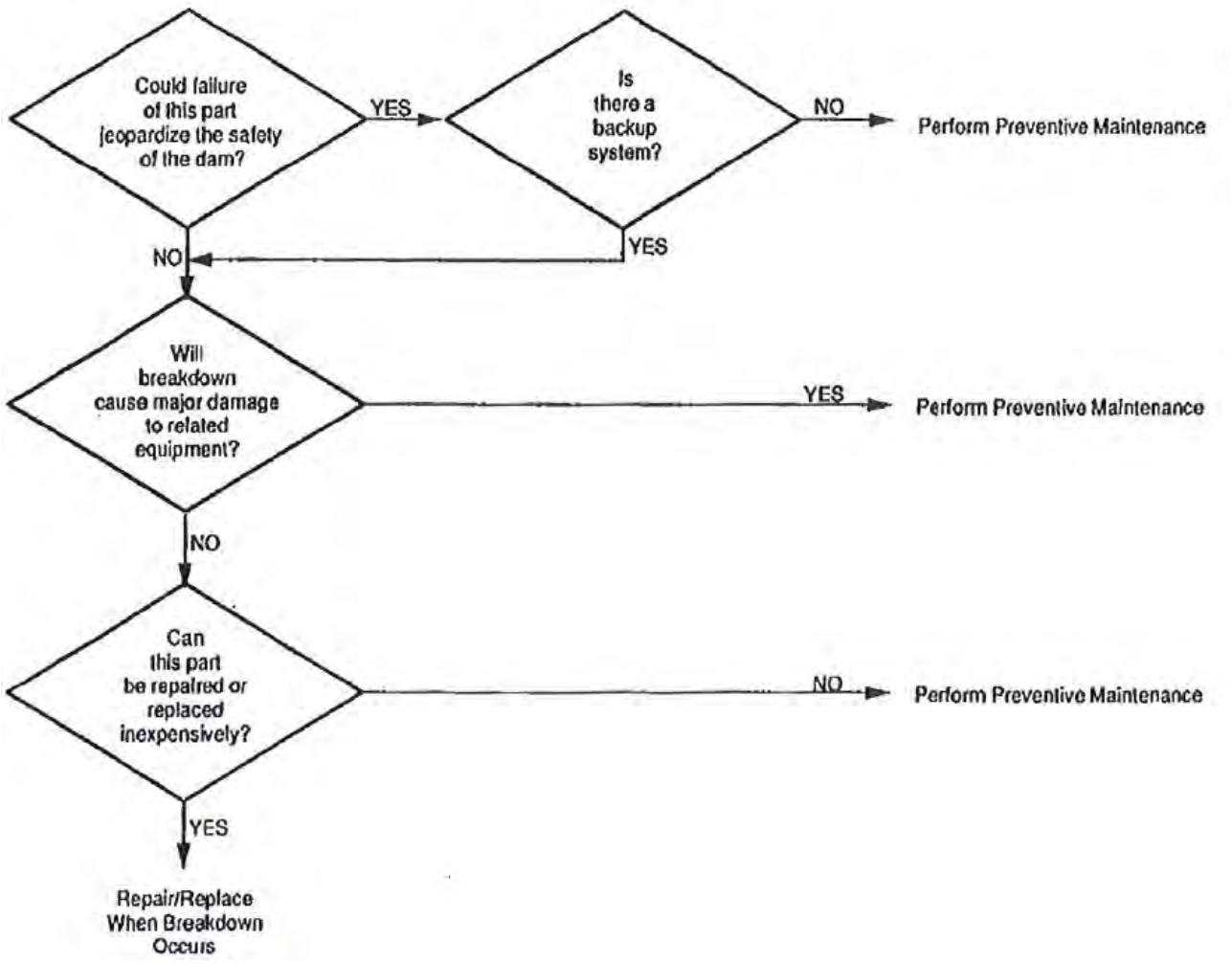
D.2.1 GENERAL APPROACH

The global process of operation and maintenance of a Hydro Power Plant is articulated through two types of actions: the preventive maintenance and the extraordinary maintenance.

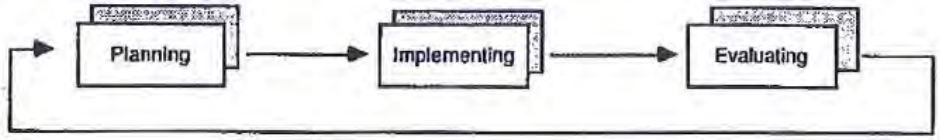
While the second one is applied in a specific case of malfunction or accident on a part of the plant, the preventive maintenance is ordinarily applied during the normal operation of the plant.

In general terms, the guiding principle of preventive/predictive maintenance is the regular and systematic application of engineering knowledge and maintenance attention to equipment and facilities to ensure their proper functionality and to reduce their rate of deterioration.
In addition to dedicated engineering, preventive/predictive maintenance encompasses regular examination, inspection, lubrication, testing and adjustments of equipment without prior knowledge of equipment failure. Preventive/predictive maintenance also provides the framework for all planned maintenance activity. The result is a proactive (rather than a reactive) environment, optimizing equipment performance and life.
It includes actions which extend the life of equipment and avoid unnecessary failures by substituting selective programmed effort for "fix it when it fails" maintenance.

The aim of any intervention is to promote cost-efficient decisions, to minimize the overall maintenance costs by means of preventive maintenance, following decision logic in terms of cost efficiency:



The conceptual process of implementing the Operation & Maintenance program is recalled here below for the benefit of understanding the general principal phases of the process.



Planning: During the planning phase, the O&M activities to be performed are identified. The frequency of each O&M activity is determined.

Implementing: During the implementation phase, the resource requirements for performing the O&M activities are identified, and the O&M activities are performed. Systems are established for monitoring and tracking O&M activities and expenditures. Finally, information is collected and records are maintained.

Evaluating: During the evaluation phase, the O&M Program is assessed. The costs and benefits of the Program are identified. Program strengths and weaknesses are identified. The assessment information is used to plan new actions for improving the O&M Program.

D.2.2 ROLES AND RESPONSIBILITY

Roles and responsibilities for the activities of operation and maintenance of the Plant are assumed to be assigned by the Plant Owner(s) to the "Plant Management Structure" (PMS), structured taking into account the Owners needs, the two Plants operation needs, and the transboundary nature of the project.

It is here recalled that monitoring inspections are to be conducted by staff trained and certified as competent in dam safety inspections; maintenance operations shall be conducted by skilled staff, and monitoring data assessments and dam safety decisions are required from qualified engineers experienced in dam safety management.

D.2.3 PROCEDURE FOR CIVIL WORKS INSPECTIONS AND MONITORING

Inspections are foreseen to be carried out periodically on dam and other plant structures, in order to:

1) carry out a DAM SAFETY MONITORING

   For the dam safety monitoring the inspections mainly consist in the periodic (typically on monthly basis) visual inspections and monitoring of the installed instruments, as described in the Operation manual of this report to which reference shall be made.

   The goal of monitoring is to confirm ongoing safe performance or to identify changes from usual performance so that corrective actions can be taken before a catastrophic failure occurs. The management of emergency events is the object of the Emergency Preparedness Plan provided in separate dedicated part of the Dam Safety Plan.

2) Identify MAINTENANCE NEEDS

   A Periodic visual inspection of all the main civil works and components of the Plant shall be conducted to check their status and identify maintenance needs. Such inspection can be combined with the one of point 1 above. The maintenance of the works will be conducted according to the results of these inspections.

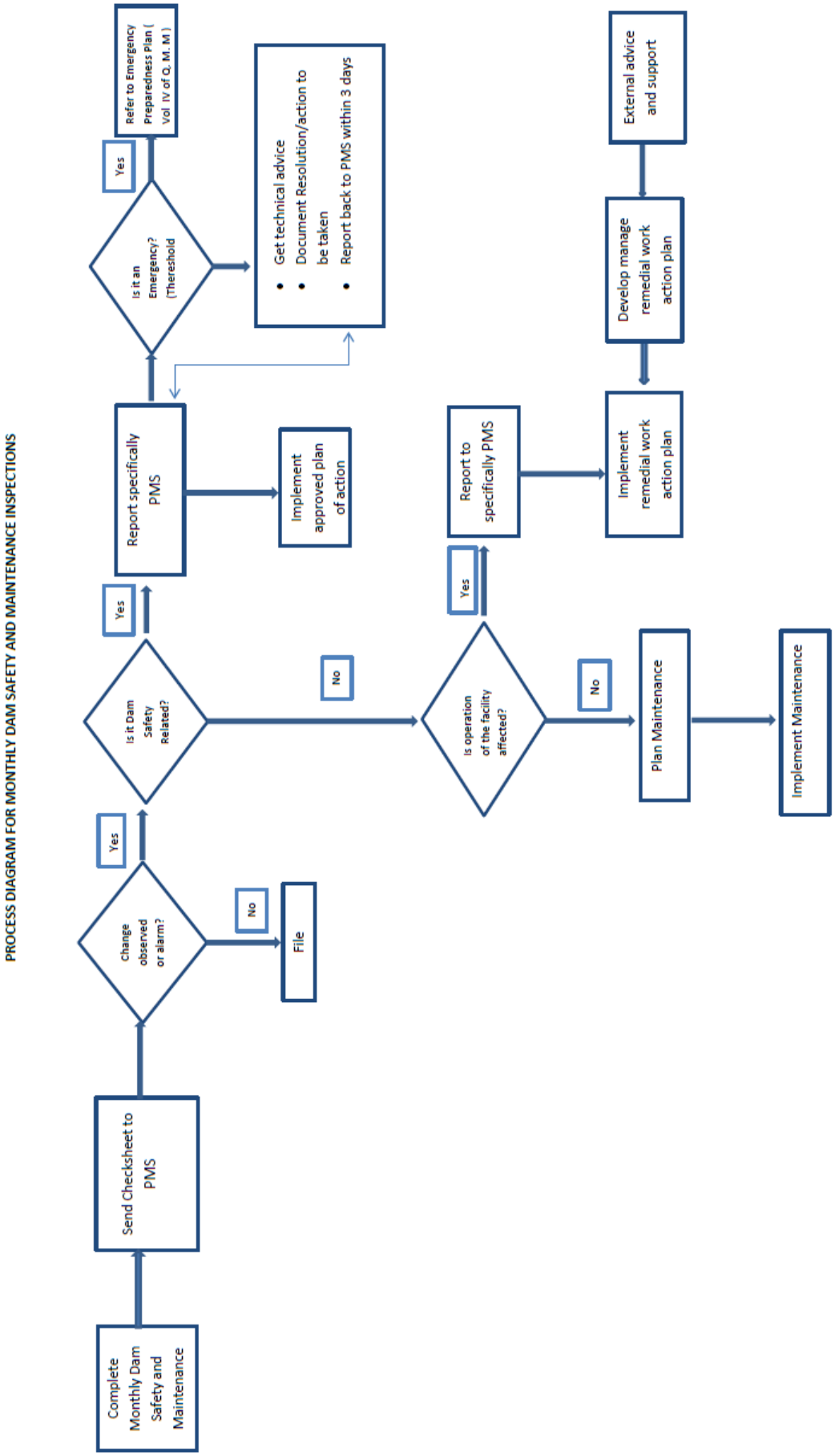
   Ordinary routine cleaning and maintenance activities are expected to be carried out systematically as far as needed to maintain in good and proper status the structures and their functionality.
   Such routine maintenance is that which can typically be scheduled on the basis of time (weekly, monthly, etc) usage (Number of cycles hours of operation, etc) or observed condition from periodic visual inspections that identify dirty, small damages, excessive wear, corrosion etc.
   Additional inspections should be carried out after significant events such as large floods or seismic events.

Specific items to be checked for each work or main component of the plant is provided in detailed lists in the following chapters of this report (each chapter dedicated to a specific work of the plant), with indication of minimum frequency required for the checks.

Operation and maintenance of all HSS and EM equipment pertinent to the dam structure will be detailed in relevant HSS and EM specific operation manuals. In the chapter D.3.14 "HYDRAULIC DEVICES AND MAIN CONTROL EQUIPMENT MAINTENANCE" at the end of this report it is however provided a table relevant to the basic guidelines for the proper inspection and testing of main hydraulic, mechanical and electrical devices which function is essential for the plant operation and dam safety. They shall be integrated with the dedicated detailed operation and maintenance manuals provided by the relevant suppliers.

Even with an effective preventative maintenance programme, there is a need to be prepared for emergency maintenance. This might include having critical spare parts, tools, equipment and trained competent staff ready in the event of an emergency.

A flowchart illustrating what to do if maintenance and / or dam safety issues are identified is included hereinafter. It can be further developed by the PMS according to his structure.


PROCESS DIAGRAM FOR MONTHLY DAM SAFETY AND MAINTENANCE INSPECTIONS
Complete Monthly Dam Safety and Maintenance
Send Checksheet to PMS
Change observed or alarm?
No
File
Yes
Is it Dam Safety Related?
No
Yes
Report specifically PMS
Implement approved plan of action
Is it an Emergency? (Thereshold
Yes
Refer to Emergency Preparedness Plan (Vol IV of O. M. M)
Get technical advice
Document Resolution/action to be taken
Report back to PMS within 3 days
Is operation of the facility affected?
No
Yes
Report to specifically PMS
Implement remedial work action plan
Develop manage remedial work action plan
External advice and support
Plan Maintenance
Implement Maintenance

As far as the first box of the above diagram is concerned, it is to be clarified that the inspection program may not necessarily be implemented for all items on a monthly basis, but according to the required frequency that will be coordinated with the dam monitoring program implemented by the Employer taking into account the prescriptions presented in this report.

Following each inspection the inspection record should be assessed by a maintenance supervisor for organizing the appropriate maintenance activity.
Records of all maintenance activities are to be kept to provide a cross reference with the initial maintenance inspection recommendation. In the last chapter of this report a format for this task, in form of a typical CHECKLIST table, is provided.
Consequent to the inspection process described here above, the implementation of the maintenance activities or other activities linked to the dam safety shall follow the general criteria outlined in next paragraphs.

Any maintenance or repair or modification action consequent to an inspection shall be duly recorded, and archived for future use and reference, for each component of the project. The operator will organize an archive in digital and paper format, divided per parts of the work and for each part, as appropriate, for type of works (concrete, E&M equipment, HSS equipment, services, masonry, finishing, etc.)
This is a need for the benefit of anyone in the future will operate the plant, and for an easy identification of any (even minor) modification or intervention occurred during the lifespan of the plant, that allow right knowledge for subsequent maintenance and repair interventions.
Record, a part the check list compilation, can include according to the need dedicated sketches or drawings, or prescriptions/instruction of a certain element supplier, or any specific action for the subsequent monitoring and maintenance activity if some change in respect to the previous status of the works/equipment occurred.

D.2.4 GUIDELINES FOR MAINTENANCE ACTIVITIES

In each of the tables reported in following chapters, dedicated to the single civil works, there are provided also, in the last column "action-remarks", some indication of actions to be taken as consequence of observed anomalies.
As far as the DAM MAINTENANCE is concerned, the observations gathered during the inspection shall be acquired at the appropriate level according to the case, and the indication of actions to be taken integrated by the consultation of the specific maintenance manual of single components or products (if available) or by recurring to specialists or consultants if necessary for not ordinary maintenance need cases.
The ultimate aim of the maintenance activities is to ensure that the plant is capable of reliably performing its operational functions with no forced outages and at the minimum maintenance cost.

For issues concerning the dam (or other major works) safety and stability, and in general as far as the outcomes of the DAM SAFETY MONITORING inspections are concerned, the above mentioned indications shall be managed within the PMS by the personnel with adequate knowledge and decisional capacity as appropriate, on case by case basis.
Whenever deemed necessary the Designer or other specialist consultant shall be consulted.
In any case, if an emergency case has to be faced, the Emergency Preparedness Plan provided in dedicated part of the Dam Safety Plan shall be applied.

The following shall be considered in the organization/ implementation of any maintenance activity:

- it is necessary for operators to be familiar with the performance of this equipment, especially if it is otherwise infrequently used.
- The timing of plant maintenance is based on routine inspections, testing and plant history, so that maintenance work is planned and effected before the risk and consequences of failure, or declined performance become incompatible with the availability, reliability and ratings required for operation.
- Essentially the maintenance regime is a combination of preventive and predictive maintenance, which minimizes the maintenance cost and the plant malfunctions.
- The plant manufacturer's maintenance instructions and recommended intervals form the initial basis for scheduling the extent and frequency of maintenance work. However extent and frequency are subject to change in the light of in service performance.
- In particular the mechanical and electrical equipment require appropriate maintenance and testing. Gates, lifting equipment and power supplies should be continuously maintained during frequent inspections and minor maintenance works, as illustrated in the last chapters of this report and in the relevant manuals of the Plant Contractor.

D.2.5 FIVE-YEARLY DAM SAFETY VERIFICATION

Five-yearly Dam Safety Verifications should include the hydraulic structure equipment essential for operation, namely all gate structures and hydraulic devices as listed in the chapter D.3.14 "HYDRAULIC DEVICES AND MAIN CONTROL EQUIPMENT MAINTENANCE".
These comprehensive inspections should be carried out by an independent expert examination team, comprising as a minimum a civil, an electrical and a mechanical reviewer, and include the following tasks:

- Comprehensive site inspection and witnessing of gate testing;
- Review of dam and gate structure design and potential failure modes;
- Review of dam monitoring data;
- Review of operations and maintenance issues and records;
- Review of emergency preparedness procedures and documentation;
- Review of personnel training record and competencies certification.

D.3 MAINTENANCE

D.3.1 RESERVOIR MAINTENANCE

Here below are resumed the principal aspects and actions required for the maintenance plan relevant to the RESERVOIR. Reservoir level monitoring and the monitoring of all topographic instruments are not included, being described in other section this report.

What indicated in the table above is indicative and preliminary and needs to be detailed and updated according to the detailed design and construction process, when it will be also available a list of detailed design or as built drawings to which reference be made for each item of the table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Reservoir shoreline	Clear and evident signals marking the shoreline, all around the reservoir perimeter.	First two years: 6 months. in case of absence of problems, from the third year: every 5 years , or in case of major flood or earthquake events.	Visual inspection of reservoir shoreline marks, all around the reservoir perimeter. If it is observed a defect in the proper and adequate identification of the shoreline for the main operation and flood reservoir levels, new marking or refurbishing of the existing one shall be carried out. In particular this activity is required in habituated lands areas.
2	Reservoir Slopes movements	Signal of movement or slides of reservoir slopes, all around the reservoir perimeter.	First two years: 6 months. in case of absence of problems, from the third year: every 5 years , or in case of major flood or earthquake events.	Visual inspection of reservoir slopes, all around the reservoir perimeter. In the dam proximity, topographic check with existing benchmarks can be implemented (see instrument monitoring system). If sliding phenomena or cracks opening on the ground are observed, they shall be monitored and, if necessary, slope stabilization measures considered (see NOTES)
3	Reservoir Slopes movements near dam	Signal of movement or slides of reservoir slopes in the vicinity of the dam.	First two years: 1 month. in case of absence of problems, from the third year: every 1 year , or in case of major flood or earthquake events.	Control of fix benchmarks coordinates, with a moveable sighting station and a theodolite or with a GPS. To be compared with the given coordinates given during construction. (see also instrument monitoring system). Measures shall be collected and filed in tabular (spreadsheets) and graphic format. Anomalies in trend or significant displacement and movements registered shall be reported. Iif necessary, slope stabilization measures reconsidered (see NOTES)
4	Floating Barrier (if present)	Damages of buoys. Rusting or damage of anchorage and steel components.	every 5 years, or in case of major flood events.	Dismantling of the floating barrier, and repair/substitution of damaged parts. Activity to be carried out with skilled people. Barge or boat required for re-installation at the end of the reparation process. Minor repairs can be conducted, if possible and convenient, without dismantling the floating barrier, directly from a boat within the reservoir.
5	Floating Barrier (if present)	Accumulation of debris or other floating elements.	First year after impounding completion: 3 months. Then every year, at the end of rainy season, or in case of major flood events.	cleaning of floating barrier by the trunks, debris or other floating elements that could be accumulate with the time. By means of small boats and racking devices accessing from dam crest or reservoir shoreline near dam (dam abutments yards)
6	Reservoir Water Cleaning	In the proximity of the spillway gates, along the dam crest, presence of trunks, debris or other floating elements that could be accumulate with the time.	First year after impounding completion: 2 months. from the following years: every year, at the end of rainy season, or in case of major flood events.	cleaning of superficial waters by the trunks, debris or other floating elements that could be accumulate with the time. By means of small boats and racking devices accessing from dam crest or reservoir shoreline near dam (dam abutments yards). Do not throw any trunk or debris through the spillway.

NOTES:
In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management Structure direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
All operations shall be done by skilled personnel, instructed and authorised by the Plant Owner(s) for the operation required.

Table 12 - Principal aspects and actions required for the Reservoir maintenance plan

D.3.2 DAM CIVIL WORKS MAINTENANCE

Here below are resumed the guidelines and actions required for the maintenance plan relevant to the DAM.

Monitoring of all instruments inside the dam are integral part of the dam monitoring and maintenance system, but they not included here, being already described in other section of the report to which reference shall be made.

The following table is mainly focused on the civil works.
What follows is indicative and preliminary, to be detailed and updated according to the detailed design and construction process, when it will be also available a list of detailed design or as built drawings to which reference be made for each item of the table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Instruments monitoring	The dam instruments monitoring is described in other section of this report, to which reference shall be made. In addition ordinary inspection and maintenance of the instruments (including the meteo station) is required: check of proper functioning of each installed instruments (lubrication, cleaning, greasing, software updating, connections, data acquisition and transmission)	1 month	Ordinary maintenance (lubrication, cleaning, greasing, software updating, check of connections) to be carried out according to the relevant maintenance manuals provided with each instrument by the supplier. Defect, malfunction or rupture of any instrument shall be recorded and appropriate action taken (see NOTES).
2	Dam drain system pumps control	Check of proper functioning of the pumps located in the downstream pits (**) check of proper functioning of pumps (lubrication, cleaning, power and control cables, start-stop signal system functions) and cleanliness of the output pipes	First two years: 1 month. in case of absence of problems, from the third year: every six months, or in case of major flood or earthquake events.	At each control of the pumps an operation of start and stop of each pump shall be done. The cleanliness of the output pipes shall be checked and eventually restored. (**) A part the ordinary cleaning, in case of any problem observed, follow the pumps maintenance manuals. Defect, malfunction or rupture of any pump shall be recorded and appropriate action urgently taken (see NOTES).
3	Galleries drain ditches Cleaning	The drain ditches shall be checked to be free from obstructions or mud or debris that can be obstacle to the flowing of the water. (**) This check applies also for pits at the side of each downstream entrance to the galleries, and for the pumps pits	First year after impounding completion: 2 months. from the following years: every year.	The drain ditches shall be always clean and free from obstructions or mud or debris that can be obstacle to the flowing of the water. As far as necessary they shall be cleaned by hand or mechanical tools (the same for the pits). (**)
4	Drains	The drains inside the dam shall be checked to be free from obstructions or calcifications that can be obstacle to the flowing of the water.	Inspection and single drain maintenance: every year.	The single drains obstructed shall be cleaned by mechanical means or high pressure water jets.

		A systematic periodic cleaning is suggested	Systematic cleaning: 10 yeas	Long term periodic washing of all the drain is suggested to contrast progressive expected occlusion.
5	Illumination	Visual control and inspection of lamps and cables functioning for both external (light posts) and internal (lamps inside galleries and control buildings) illumination systems	1 year	Ordinary maintenance of all illumination system and electrical parts shall be done, as needed.
6	Concrete surfaces	Visual control and inspection of the status of all concrete surfaces. In particular it shall be checked: - Presence of cracks - Water percolations - Possible exposed reinforcement In particular, as far as dam is concerned, this check shall be carried out inside the galleries, on dam downstream face and, as far as possible on the upstream face). Special attention shall be paid to the crest spillway surface. Analogue prescription applies for all the civil structures of the project, not only to the dam.	1 month. In any case always after exceptional flood or earthquake event.	Observed anomalies, if any shall be recorded, monitored and appropriate actions taken (see NOTES). If it is ascertained, by the Plant Management direction, that the defects registered do not involve problems for the stability and safety of the structure, ordinary maintenance activities can be carried out to repair such defect. On the contrary case, actions shall be defined on case by case basis consulting a specialist. Whenever reinforcement appears exposed, following measures shall be taken: - concrete shall be locally demolished, - reinforcement brushed and cleaned from oxidations and dusts, - reinforcement protected against corrosion, - completely saturate with water the zone to be reconstructed, waiting evaporation of water in excess, -prepare a grout with water+ high resistance cement+ selected inert+ synthetic fibres admixtures gently mixing to incorporate air also, - apply no shrinkage grout, for thickness of no more than 3cm per layer, than finishing of the surface.
7	Control building, access buildings civil works finishing	Visual control and inspection of the status of all concrete or metal surfaces, structures and finishing pertaining to the buildings and civil works structures. In particular it shall be checked: - Presence of cracks in the concrete - Water percolations - Possible exposed reinforcement - leaks from the roofs - finishing deterioration - paintings deterioration - damages to tiles, if present - functioning of doors and windows - rusting of metal structures - status of steel girders, guardrails, parapets, lamp posts, stairs, ladders - status of the gutters and drains, that shall be free from obstructions. For the dam this check includes the Dam Control Building, Diesel Generator	6 months In any case always after exceptional rainstorm or earthquake event.	Ordinary maintenance of all concrete surfaces, metal works and finishing shall be done, as needed. Steel girders must be repainted when rust will appear. Repair of exposed concrete can be conducted as described at point 6 above). If it is ascertained, by the Plant Management direction, that the defects registered involve problems for the stability and safety of the structure, actions shall be defined on case by case basis consulting a specialist.

		Building, Cable shaft access Room on dam crest, Spillway gates servomotors rooms on dam piers, and other access structures also on dam crest). Similar prescription applies for all the civil structures of the project.		
8	Lift inside cable duct (if present)	Check of lift functioning. Check of proper lubrication of the mechanism, according to the supplier maintenance manual	1 year In any case always after exceptional earthquake event.	Ordinary maintenance according to the supplier maintenance manual. Repair in case of rupture, as far as to maintain it available for access inside the dam at any moment.
9	Lifting structure for access/repairs of pumps, MLO gates, and ancillary works on dam downstream face	Visual control of the status of metal structure and its anchorage Check of functioning of lifting device.	1 year, and in any case always after exceptional earthquake event. Every 10 years the test of lifting device shall be done with a load equivalent to the pump weight.	Ordinary maintenance of metal works and finishing shall be done, as needed. Any damage to the lifting device shall be reported to the Plant Management direction for action of repair.
10	Fences and Gates	Visual control and inspection of the status of fences and gates In particular it shall be checked: - rusting of metal structures - painting deteriorations - damages to fences or their anchorage - functioning of locking of galleries doors - functioning and status of restricted areas gates	1 year In any case always after exceptional rainstorm or earthquake event.	Ordinary maintenance of all metal works and finishing shall be done, as needed, to maintain them and to allow full access to authorized people inside the restricted areas.
11	Dam access roads	The efficiency and cleaning of dam access roads (including the dam crest road) and their drain ditches shall be checked.	Six months In any case always after exceptional rainstorm or earthquake event.	Ordinary maintenance and cleaning of all roads pavement and deep cleaning of all the drain system is required to maintain the functionality of the roads, metal works and finishing shall be done. Local repair of any damages is required, as needed.
12	Protection of the slopes at yards and access roads	It shall be checked the efficiency and status of the support and protection system of the slopes around the dam (in particular the excavated slopes around the abutments yard and of the access roads). In particular it shall be checked: - Defective anchors - Break of rockfall protection nets or barriers - Filling of debris of rockfall protection nets or barriers (if present) - cracks or damages to the shotcrete - Important or extended leaks from drains. - sing of movements of the excavated front - rockfalling or slides events.	Six months In any case always after exceptional rainstorm or earthquake event. Periodic cleaning of the rockfall protection nets or barriers required.	Any observed damage or anomaly shall be measured, documented and reported in all details. In case of instable wedges to be scaled, this shall be done by skilled people. Extended areas of damaged shotcrete shall be cleaned and repaired. Damaged bolts shall be replaced. In case of continuous important or extended flow of water is observed coming out from drainage holes, the flow shall be monitored (quantifying the flow and reporting it on a spreadsheet). Than a specialist designer shall be consulted. Defect, malfunction or rupture of any anchoring or protection system, as well as any sign of excavated front movement, shall be recorded and appropriate action taken (see NOTES).

				Periodic cleaning of the rockfall protection nets or barriers (where present) required.
13	Dam crest road bridges beams and spillway main crane railways beams.	All steel parts (if any) of the main deck structure shall be periodically inspected by skilled personnel. Special attention shall be paid in checking the bolted and welded junctions, and the integrity of the superstructures as well as presence of rust or paint damages.	First visit 1 year after construction than frequency of controls to be defined, in any case no more than every 5 years	Eventual anomalies or damages in steel structure and junctions shall be documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see NOTES).
14	Dam crest shock absorbers (if present)	1. Visual control of the general status of the device. 2. Control on the compatibility of the stroke or the rotations occurred with the ones foreseen by the device. 3. Control of the bolts tightening and of the connections. 4. Control of any oil leakage. 5. Check of external cleaning and possible presence of rust. 6. Control that the sliding systems are not seized-up or ruined 7. Verify the integrity and efficiency of stem protection.	First control (only check indicators 1,2 and 3) after six months from the installation. Then full check (7 check indicators) every 2 years or in case of seismic event.	Eliminate possible dust and/or encrustation. Remove and restore possible rusted points. Contact the supplying firm in case of: - Oil leakage - the sliding systems are not seized-up or are ruined - presence of rust in unaccessible zones. Every 10 years it is suitable a verification at the workshop, by removing a number equal to 5% of the devices installed, with a minimum of one.
15	Crest road bridges bearings and spillway main crane railways beams bearings (as far as present and applicable) periodic check	Bearings shall be checked periodically, in order to verify: - Correct positioning of the structures and status of bearing bed and anchorage. - Absence of movement or anomalous deformations Capacity of the bearing to assure the consented movement of the structure, verifying the displacement of the bearings mobile parts during different seasons. - Absence of ruptures - The good conservation of the devices foreseen against corrosion and against dust - The uniformity of the contact, as foreseen in the project. - The maintain of the designed geometry (parallelism and planarity of contact surfaces) - Eventual anomalies or cracks in structures near the bearings, that could be induced by a problem to the bearing.	2 years	Eventual anomalies in the bearings or cracks in the concrete structure near the bearings shall be monitored, documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see NOTES) Bearings must be checked and, when necessary replaced with new ones. For frequency of controls, maintenance operations and all details for replacing see relevant maintenance manual provided by the bearing supplier. For maintenance or replacing of part or all the bearing, the relevant bridge deck shall be lifted for this operation. The jacks for lifting operation shall be inserted in dedicated holes and shall have the design load and characteristics given in design drawings.

Table 13 - Principal aspects and actions required for the Dam civil works maintenance plan

(**) The following shall be considered in organizing such maintenance operation:

- Consideration should be given to monitoring the depth and type of sediment at the gallery weirs before each gutter cleaning, rather than just removing and discarding it. This should be done on a regularly scheduled basis (say every 2 weeks during reservoir impounding).
  Samples of the sediment should be analysed on occasion for grain size, plasticity, pH, and composition. This information can be very useful if any piping or erosion starts in any of the foundation drains. The pH can be used to indicate if some of the seepage is coming from internal drainage of the dam itself, picking up calcium hydroxide (basic) on the way. Consideration should also be given to using a hand held "pistol" type temperature indicator to check the temperature of water coming from foundation drains. It is recommended to also keep simple method of documentation for easier consultation, just writing the date and temperature on the wall next to the drain with a pencil. If the temperature changes, the date and new temperature should be noted. This could, for example, indicate that water from the reservoir is migrating to a hot spring area if the temperature of water from those drains decreases as the reservoir is raised.

- Consideration should be given to recording the volume of water pumped per week or month from the gallery sumps. Seepage flows are measured using weirs in the gallery gutters, but the pump records can be used as a check or verification.

D.3.3 MIDDLE OUTLETS MAINTENANCE

Here below are resumed the guidelines and actions required for the maintenance plan relevant to the MIDDLE OUTLETS.

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the following table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Middle Level Outlets Drains	Any specific drains running around the middle outlet steel lining, exit in nearest the dam gallery, in correspondence of the two middle outlets. The water venues through these drains shall be monitored as prescribed and done for the other drains of the dam. The drains shall be checked to be free from obstructions or calcifications that can be obstacle to the flowing of the water.	Inspection and single drain maintenance: every year. Systematic cleaning: 10 years	Anomalous records shall be reported to PMS. The single drains obstructed shall be cleaned by mechanical means or high pressure water jets. Long term periodic washing of all the drain is suggested to contrast progressive expected occlusion.
2	Gates maintenance platform	Maintenance platforms and devices envisaged on the dam, outdoor, shall be checked. Check of good status of the steel, check of possible presence of rusting is required. Periodic test of platform opening and closing mechanism (as far as present and applicable) is required.	Every year.	The platform/device shall be maintained efficient by means of ordinary maintenance activity (including mechanism greasing as necessary) and periodic testing of functioning.
3	Steel lining	Check of alignment, status of weldings and joints, cavitation effects	Every 5 years, or in any case of emergency or prolonged use of MLO.	Empting the MLO conduit by closing the upstream bulkhead is required for this operation. Any anomaly shall be duly recorded and signalled (see NOTES).
4	Recess for d/s gate servomotors pumps	From dam d/s berm it is possible to get access to the chamber housing the servomotors for d/s gate operation. Check of good status and functionality of the chambers structure, gates, drains and relevant accessibility is required. Check shall be extended to the cables corridors.	Every year.	Servomotors shall be used and maintained as prescribed by the specific gates operation and maintenance manual. Accessibility to their chambers shall be maintained efficient by means of ordinary maintenance activity.
5	Sand disposal valves, by-pass valves, other auxiliary hydraulic devices.	Check of good status of the valves and hydraulic devices, of their functioning and of the accessibility to their chambers is required.	Every year.	Valves shall be used, during or after any use of middle outlet, as prescribed by the specific gates operation and maintenance manual. Accessibility and functioning of the valves shall be maintained efficient by means of ordinary maintenance activity and periodic testing of the valves functioning.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 and Pr.7 in table reported in chapter D.3.2 shall be applied.

Table 14 - Principal aspects and actions required for the Middle Outlets maintenance plan

D.3.4 SPILLWAY MAINTENANCE

Here below are resumed the guidelines and actions required for the maintenance plan relevant to the SPILLWAY.

The following table is mainly focused on the civil works (Gates and other EM and HSS equipment Operation and Maintenance manuals are issued separately).

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the following table.

The prescriptions about civil work regarding the beams, bearings and shock transmitter units possibly present on the top of the spillway piers are provided in Pr.13 "Dam crest shock absorbers", Pr.14 "Crest road bridges bearings and spillway main crane railways beams bearings periodic check" and Pr.15 "Dam crest road bridges steel beams and spillway main crane railways beams" in table reported in chapter D.3.2, to which reference shall be made (they are not reported again here).

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Spillway radial gates tendons	Spillway radial gates piers have tendons in their body. They do not require in principle any special maintenance, being fully greased and vipled as protection against corrosion and inserted in a sleeve pipe fully embedded in concrete. However the space between tendon and sleeve pipe is not grout injected, and this allows possible intervention for possible re-tensioning operations. It is therefore recommended, within the ordinary inspections to the civil works, to carry out specific visual check of: - possible presence of cracks or sign of deformation that might appear on the piers. - Misalignment of the radial gate in respect to its lateral guides. - Malfunctioning on opening or closure operation of the radial gate.	Ordinary inspections on monthly basis, or after any relevant flood event. Direct measures of tendons tension is recommended to be carried out 1 year after initial pulling, then 5 years after initial pulling, then every 10 years. In any case after any flood or earthquake event.	In case signs of piers deformation are detected or possible gates mulfanctioning indicating possible tendons detensioning, they shell be duly reported to PMS and, upon consultation of expert, possible intervention of re-tensioning of one or more tendons can be organized and implemented (see NOTES). In such case the following aspects are to be considered: Wether access to the tendons heads is possible from a dedicated shaft located inside the pier The head of the tendons are typically protected with tixotropic gel (that shall be restored at the end of any intervention). Tensioning shall be carried out by adequate jack controlled by a standard multi-cables control system. A regulating ring nut can be foreseen on the head of the tendon anchor, allowing an easy intervention with the jack

				for tensioning and de-tensioning operation. This special maintenance operation, as well as other possible ordinary maintenance operations (lubrication, cleaning, greasing, software updating, check of connections) are to be carried out according to the relevant maintenance manuals provided by the tendons supplier.
2	Spillway drains	The drains pipes of the drain system laying under the spillway chutes shall be checked to be free from obstructions or calcifications that can be obstacle to the flowing of the water.	Inspection and single drain maintenance: every year or after important or prolonged flows passing through the spillway. Systematic cleaning: 10 years	The single drains obstructed shall be cleaned by mechanical means or high pressure water jets. Long term periodic washing of all the drain is suggested possible occlusion.
3	Lateral walls ceiling and aerators ducts	Visual check of good status and possible movement of the structures	Every 5 years or after important or prolonged flows passing through the spillway.	Damaged or moved elements shall be replaced and/or properly fixed. Aerator conduits if obstructed or damaged shall be cleaned or repared as needed.
4	Chute finishing	Visual check of good status of the spillway chutes slabs, including check of: • Presence of fissures or cracks; • Detachment of finishing concrete layer at the contact with lateral walls and a the chute slabs extremities • Damages (erosion, cavitation effects) on concrete surfaces	For first two years every year, then every 5 years, or in any case after important or prolonged flows passing through the spillway.	Any anomaly shall be duly recorded and signalled, possibly associated with measures of spillway discharge, meteo conditions, spillway drains outflows (see NOTES). Repairing intervention, if needed shall be properly designed (for instance making use of suitable products to repair damages induced by water passing at very high speed). and conducted by skilled people, in safe conditions.
5	Radial gates lateral sealing	Visual check of possible leaks on the radial gates sides when gates are closed and reservoir is at maximum operating level.	For first two years every year, then every 5 years, or in after important or prolonged flows through the spillway.	Any anomaly shall be duly recorded and signalled (see NOTES). Deeper checks (such piers alignment, water discharge and possible dependence from meteo conditions) to be prescribed on case by case basis.
6	Piers upstream face, stoplogs grooves and chute warterways	Visual check of presence of trunks and debris transported by the water near the gates and stoplogs grooves when gates are closed. Visual check of possible damages to the concrete or presence of trunks along the chutes after flood passage through the spillway.	Every 5 years, or in any case after important or prolonged flows passing through the spillway.	Any anomaly shall be duly recorded and signalled (see NOTES). Ordinary maintenance of civil works is required. Debris or trunks invading the stoplogs grooves or the chutes shall be immediately removed.

7	Piers joints.	Visual check of presence of infilling or debris within the joints. Upstream of the water-stop alignment, the water shall be free to enter within the space of the vertical joints.	Every year, or in any case after important or prolonged flows passing through the spillway.	Any anomaly shall be duly recorded and signalled (see NOTES). Ordinary maintenance of civil works is required. Debris or trunks obstructing the joints upstream of the water-stops alignment shall be cleaned/removed.
8	Radial gates servomotors pumps rooms	From dam crest it is possible to get access to the chambers housing the servomotors for radial gates operation. It is required a periodic visual check of the good status and functionality of the chambers and cable bridge structures and their accessibility.	Every year.	Servomotors shall be used and maintained as prescribed by the specific gates operation and maintenance manual. Accessibility to their chambers and to the cable bridge shall be maintained efficient by means of ordinary maintenance activity.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.
4) For shock transmitters units and bearings on beams located on the top of the spillway piers, see specific prescriptions provided in table reported in chapter D.3.2.

Table 15 - Principal aspects and actions required for the Spillway maintenance plan

D.3.5 PLUNGE POOL MAINTENANCE

The pool is designed to safely withstand floods events.

The pool, and the concrete structures protecting the dam downstream toe are conceived as sacrificial barriers and are not meant to be fully stable and not subject to erosion or scouring in the long term. They are conceived as first line of defense to protect from scouring the permanent works such the dam and the power house.

A part the ordinary maintenance of the abutments slope stability supports, important to guarantee in the long term the accessibility to the dam (see prescriptions on relevant DAM chapter), the plunge pool area doesn't require specific operation and maintenance actions.

At this level of the design, no specific actions are therefore foreseen in the submerged portion of the pool during the normal operation of the plant.

Some scouring is expected to occur within the pool, and material (rock blocks) to remain in its bottom.

It has to be reminded that most of the pool will be permanently under water, and inspection and possible maintenance actions implies the use of a barge or its dewatering by means of pumps. This means that such actions shall be programmed and planned in advance and in any case shall be carried out only when strictly necessary.

It is however recommended to check its behavior and stability in the case of specific exceptional flood or extraordinary spillway discharge event, especially as far as the aspects affecting the proper functioning of the permanent structures of the plant (Power House and Dam). Such checks are described in the following table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Slopes stability	Visual check of: • status of shotcrete, bolts, drainages and drain ditches foreseen on benches • potential instable wedges. • Signal of movement or slides of slopes. • Debris accumulation on benches. • Check of possible crack openings or fissures or deterioration of concrete surfaces. The visual check will be ordinarily conducted on the accessible portions of the excavations (upper portion of the excavated fronts), as far as possible extending the observation to the submerged parts (for instance during dry season or possible stop of the plant).	After first spillway operation. After first Middle Outlets operation. Then yearly, preferably at the end of rainy season. Additionally in case of an exceptional flood event.	Any anomaly shall be duly recorded and signalled. Deeper checks (and check of possible dependence from meteo conditions) to be prescribed on case by case basis. If sliding phenomena or cracks opening are observed, they shall be monitored and, if necessary, slope stabilization measures reconsidered (see NOTES). Accumulation of debris on accessible benches shall be removed.
2	Upstream concrete structures (dam d/s toe protection structures)	Visual control and inspection of all concrete surface, as far as visible.	After first spillway operation. After first Middle Outlets operation. Then only in case of an exceptional flood event.	Any anomaly shall be duly recorded and signalled, possibly associated with measures of spillway or MLO discharge and meteo conditions (see NOTES). Repairing intervention, if needed shall be properly designed and conducted by skilled people, in safe conditions.
3	Excavation profile	It shall be checked the profile of the pool bottom, with the aim to check if there is no risk of damages for the permanent civil works (PH and DAM) and that it is not compromised the plant energy production. This check can be done firstly visually, checking if there is accumulation of eroded material out of the pool. In this case, and in any case on 10 years period basis, a bathymetric survey is required.	After first spillway operation. After first Middle Outlets operation. In any case of an exceptional flood event ($Q>6500 m^3/s$). Recommended in any case every 5-10 years (depending on Spillway and middle outlets use).	Any anomaly shall be duly recorded and signalled, possibly associated with measures of spillway or MLO discharge and meteo conditions (see NOTES). Repairing intervention, if needed shall be properly designed and conducted by skilled people, in safe conditions.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 16 - Principal aspects and actions required for the Plunge Pool maintenance plan

For the check of concrete surfaces reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the previous table.

D.3.6 INTAKE GATES STRUCTURE and relevant upper yard MAINTENANCE

What follows is referred to one structure but applies for both left and right Power Waterways.

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the following table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Yard slopes protection devices	Visual check of the visible (not submerged) portions, in particular check of: • status of shotcrete, bolts, drainages and drain ditches foreseen on benches • potential instable wedges. • Signal of movement or slides of slopes. • Debris accumulation on benches. • Check of possible crack openings or fissures or deterioration of concrete surfaces. In case of continuous flow of water is observed coming out from drainage holes, the flow shall be monitored (quantifying the flow and reporting it on a spreadsheet).	After first Power waterways operation. Then yearly, preferably at the end of rainy season. Additionally in case of an exceptional heavy rain or earthquake event , or Power Waterways dewatering.	Any anomaly shall be duly recorded and signalled. Deeper checks (and check of possible dependence from meteo conditions) to be prescribed on case by case basis. If sliding phenomena or cracks opening are observed, they shall be monitored and, if necessary, slope stabilization measures reconsidered (see NOTES). In case of instable wedges to be scaled, the scaling shall be done by skilled people. Extended areas of damaged shotcrete shall be cleaned and repaired. Damaged bolts shall be replaced. Accumulation of debris on accessible benches or any obstruction of drain ditches or pipes shall be removed. Than a specialist designer shall be consulted. Ordinary maintenance and cleaning of the drain ditches, as needed.
2	Finishing of the tunnel lining downstream of the gates	Visual check of good status of the concrete surface, including check of: • Presence of fissures or cracks; • Detachment of finishing concrete layer at the contact with lateral walls and along the transition downstream and upstream of the gates. • Damages (erosion, cavitation effects) on concrete surfaces or near joints	In case of Power Waterways dewatering.	Any anomaly shall be duly recorded and signalled, possibly associated with measures of spillway or MLO discharge and meteo conditions (see NOTES). Repairing intervention, if needed (for instance making use of suitable products to repair damages induced by water passing at very high speed) shall be properly designed and conducted by skilled people, in safe conditions.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 17 - Principal aspects and actions required for the Intake gate structure maintenance plan

The above applies for gate shafts concrete structures (yard, exposed structure and upper control building).
Other prescriptions for maintenance inspection and operation of such structures are provided in the table here below. For the maintenance of gates and all EM and HSS equipment, see relevant section.
Of course, for this structure, the portion that during normal operation of the plant is under water can be inspected only in the case of exceptional dewatering of the Power Waterways, that, unless for other exceptional needs, is recommended to be carried out every 10 years.
During first impounding it is recommended to either clean the trash racks when water is raising at intake towers levels and trunks might obstruct them, or protect them by floating barrier temporary installed in front of them.

D.3.7 POWER TUNNEL MAINTENANCE

What follows is referred to one structure but applies for both left and right Power Waterways.

During the operation of the plant the tunnel is not accessible. No special operation and maintenance needs are foreseen for the civil works of the tunnel.

Power waterways can be inspected by dewatering the tunnels (by closing the intake gates).
The power waterways dewatering (and subsequent filling) procedures are described in relevant paragraph of operation manual section, to which reference is made.

Such operation can be decided in case of need of inspection of the tunnel (for instance decided in case of important leakages detected in the drain, or in any case recommended after 10 years of tunnels operation), instructed and authorized by the Plant Owner consulted by design specialist.

The inspection of the tunnel, shall be programmed in advance to allow to manage the consequences of a long period of plant stop and long period of water discharge through the spillway/middle outlet.
It shall be carried out by skilled personnel only, instructed and authorized by the Plant owner for the operation required, consulting a design specialist.

In case of such extraordinary event of inspection, the following operations can be envisaged as far as the civil works are concerned:

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Main Tunnel lining check	Complete dewatering of the tunnel, then visual check of cast concrete: • status of lining (aging, cracks, roughness, local defects check); • Joints check; • External waterstops (alignment, fixing elements, possible detachments, possible water percolations around the waterstops) • Eventual presence of water percolations; • record and check of settlements or displacement movements. • comparison of the values measured during previous inspections. At the first inspection the comparison shall be made with the records made at the date of completion of the tunnel construction. • Presence of materials and debris inside the tunnel shallbe checked. Possible correlation with defects detected during the tunnel concrete lining inspection shall be carried out.	10 years, or in any case of abnormal water leaking record in the drain tunnels or surrounding rockmass. On the upstream portion only in case of need.	In case of observed damage to extended areas of lining a monitoring and mapping shall be performed by skilled people (see NOTES). In any case all anomalies, instable phenomena and presence of water percolation shall be recorded and appropriate actions taken (see NOTES)), to be defined on case by case basis. In case of presence of debris they shall be removed. The material found shall be analysed to identify its provenience (if it comes from rock , from river or from concrete lining elements). Than possible appropriate actions taken (see NOTES).
2	Steel lining check (For any portion of the tunnel lined with steel)	Whenever the waterway is emptied, check of the integrity and alignment of the welded junctions, and check of possible presence of cavitation zones, rust or paint damages on the steel lining. Check of integrity at intersection with concrete section. Check of circularity and good alignment of the steel lining sections. Such checks shall be done visually in any case, and with the aid of instruments as far as necessary to acquire all the information.	Whenever tunnel is emptied.	Eventual anomalies or damages in steel structure and junctions shall be documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see NOTES), to be defined on case by case basis.
3	Access tunnels supports check	Visual check of: • status of lining (shotcrete and or bolts or ribs); • eventual presence of accumulated debris along the tunnel, as consequence of failure of potential instable wedges; • Eventual presence of water percolations	Every six months (at the end of dry and wet seasons) during impounding, than every 1 year.	In case of observed damage to extended areas of lining a monitoring and mapping shall be performed by skilled people (see NOTES). Than the area shall be scaled and cleaned and shotcrete or bolts applied again. In any case all anomalies, extended instable phenomena and presence of water percolation shall be recorded and appropriate actions taken (see NOTES).
4	Access tunnels portals and relevant gates.	Visual control and inspection of steel and concrete surface. Gates lock opening and closure test shall be done every year.	Every six months (at the end of dry and wet seasons) during impounding, than every 1 year. After 5 years, every 5 years.	In case of corrosion or damage to steel gate painting against corrosion shall be done, if possible, or alternatively the part shall be substituted. For actions on concrete structures, see NOTE 3. Ordinary maintenance of all concrete surfaces, metal works and finishing shall be done, as needed.

NOTES: 1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted. 2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required. 3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.				

Table 18 - Principal aspects and actions required for the Power Tunnel maintenance plan

Operation and maintenance of valves, gates, grids, relevant cranes and other HSS and EM equipment and instruments, located along the power waterways, are described in relevant section.

D.3.8 SURGE SHAFT MAINTENANCE

What follows is referred to one structure but applies for both left and right Surge shafts.

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the following table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Surge shafts yard slopes protection devices	Visual check of the visible (not submerged) portions, in particular check of: • status of shotcrete, bolts, drainages and drain ditches foreseen on benches • potential instable wedges. • Signal of movement or slides of slopes. • Debris accumulation on benches. • Check of possible crack openings or fissures or deterioration of concrete surfaces. In case of continuous flow of water is observed coming out from drainage holes, the flow shall be monitored (quantifying the flow and reporting it on a spreadsheet).	After first Power waterways operation. Then yearly, preferably at the end of rainy season. Additionally in case of an exceptional heavy rain or earthquake event , or Power Waterways dewatering.	Any anomaly shall be duly recorded and signalled. Deeper checks (and check of possible dependence from meteo conditions) to be prescribed on case by case basis. If sliding phenomena or cracks opening are observed, they shall be monitored and, if necessary, slope stabilization measures reconsidered (see NOTES). In case of instable wedges to be scaled, the work shall be done by skilled people. Extended areas of damaged shotcrete shall be cleaned and repaired. Damaged bolts shall be replaced. Accumulation of debris on accessible benches or any obstruction of drain ditches or pipes shall be removed. Than a specialist designer shall be consulted. Ordinary maintenance and cleaning of the drain ditches shall be done, as needed.
2	Steel parts	The external steel ladder and fences shall be periodically inspected by skilled personnel.	First visit 1 year after construction than	Eventual anomalies or damages in steel structure and junctions shall be documented and reported in all details, where possible quantifying the

		Special attention shall be paid in checking the bolted and welded junctions, and the integrity of the superstructures as well as presence of rust or paint damages.	frequency of controls to be defined, in any case no more than every 10 years	extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos, than appropriate actions taken (see NOTES). Ordinary maintenance of all steel parts shall be done, as needed.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 19 - Principal aspects and actions required for the Surge Shaft maintenance plan

The above applies for surge shafts concrete structures (yard, shafts below and above ground), other prescriptions for maintenance inspection and operation of such structures are provided in the table here below. For the maintenance of gates and all EM and HSS equipment, see relevant section.

Of course, for this structure, the portion that during normal operation of the plant is under water can be inspected only in the case of exceptional dewatering of the Power Waterways, that, unless for other exceptional needs, is recommended to be carried out every 10 years.

D.3.9 PENSTOCKS MAINTENANCE

What follows is referred to one structure but applies for both left and right Penstocks.

No specific maintenance is foreseen for Manifold and Penstocks steel lined conduits, being inside the rock and embedded in concrete.
Whenever for an exceptional event one or both the power waterways are emptied (see also prescriptions reported in the operation manual section), the general check of the integrity and alignment of the bolted and welded junctions, and the check of possible presence of cavitation zones, rust or paint damages can be conducted, as shown in the following table.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Steel parts	Whenever one or both the power waterways are emptied, check of the integrity and alignment of the bolted and welded junctions, and check of possible presence of cavitation zones, rust or paint damages on the steel lining.	Whenever tunnel is emptied.	Eventual anomalies or damages in steel structure and junctions shall be documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see NOTES).
NOTES: 1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted. 2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.				

Table 20 - Principal aspects and actions required for the Penstock maintenance plan

D.3.10POWER HOUSE MAINTENANCE

What follows is referred to one structure but applies for both left and right Power Plants.

The general rules for the Power House civil works maintenance are resumed in the following table.
IMPORTANT REMARKS FOR STRUCTURE MONITORING:

A) Usual ordinary maintenance and cleaning of civil works and architectural components is required for the functionality and durability of the Power House, as well as for all the other civil works structure, and it shall be therefore considered and planned as a part of the plant operation program.
B) Another aspect important to be periodically monitored is the bathymetry of the Tailrace Hole.
   Whenever in this stretch of the river important sedimentation or rock elements accumulation would occur, this might have possible negative consequences for the energy production.
   On the contrary, local deep erosion and scouring at the base of the Power House structure might compromise its stability.
   In principle scouring is not expected here, and also sedimentation should not occur during the normal operation of the plant. Very exceptional floods discharged through the spillway might imply some transfer of material from plunge pool to tailrace hole.
C) Any restrictions for the loads on civil structures, as well prescription for loading areas, indicated in the detailed design shall be considered when programming maintenance operations.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	All reinforced concrete structures	Visual control and inspection of concrete surfaces of all structures (included upper and lower tank and diesel and GCB buildings)	Every year, or in case of exceptional flood, heavy rain or earthquake event	See NOTE 3). The first prescription about civil work regards the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, for which it shall be applied the prescriptions that are provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter 0 " DAM CIVIL WORKS", to which reference shall be made (they are not reported again here).
2	Steel parts	All steel parts shall be periodically inspected by skilled personnel. Special attention shall be paid in checking the bolted and welded junctions, and the integrity of the superstructures as well as presence of rust or paint damages.	First visit 1 year after construction , than frequency of controls to be defined, in any case no more than every 10 years	Eventual anomalies or damages in steel structure and junctions shall be documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see Notes).
3	Control building and yard area finishing and architecture	Visual inspection of all the architectural and finishing elements inside and outside the Power House, such as: fence, illumination posts, illumination system and lamps, windows, doors, pavements, paintings, access roads, external yard finishing, pedestrian walks, gutters, ditches, cable trenches, handrails, steel ladders and platforms, ancillary works. The inspection shall be carried out by skilled people, able to identify possible defects that may require ordinary or extraordinary maintenance works.	Every year, or in case of exceptional flood, heavy rain or earthquake event	See NOTE 3). Possible wearing out, damages or defects on architectural elements shall be recorded and appropriate actions prospected to the Plant Owner for his action (see NOTES). Ordinary cleaning and maintenance of fence, roads, illumination system, frames, steel works, platforms, ancillary works and furniture shall be done, as needed. All sliding and rolling elements shall be maintained suitably lubricated (for instance the wheels of the cover structure on the erection bay roof opening).
4	Joints	Visual check of the good status of the joints and joints cover, inside and outside (on the roof) of the Power House.	Every year, or in case of exceptional flood, heavy rain or earthquake event	Eventual anomalies or damages of the joints shall be documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see Notes)
5	Water system and fire fighting system	Visual inspection of all the water system (tanks, pipes, pipelines, valves, roof gutters, taps and all fittings) inside and outside the Power House building. Water system is described in relevant report. Test of functioning of all pumps located in the lower tank chamber.	1 year Differently only in case specific defects or leaks on the hydraulic system are observed,	Eventual filtration, damages or cracks on tank structure shall be recorded and appropriate actions taken (see Note). Exposed reinforcement bars shall be treated as indicated at point 1. On off and functioning tests on all pumps, according to the relevant supplier operation and maintenance manual.

		Test of fire fighting system as specified in EM operation and maintenance manual. Check of potabilizer.	or in case of fire event.	Every year lamps of UV potabilizer device and filters must be changed. Ordinary maintenance and cleaning of pipes, pipelines, valves, roof gutters, taps and all fittings to be done periodically, as needed.
6	External drainage system	All around the Power House area (on yards, road and slopes) visual check of: - Drainage pits - Ditches - Pipes Ordinary maintenance and cleaning of ditches and gutters shall be carried out, especially during the rainy seasons.	1 year Always after very heavy rainfall event occurred.	In case of big damages to any drainage structure is observed, it shall be documented and reported in all details, where possible quantifying the extension of the damage in a spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see Note); for damages to reinforced concrete see relevant point 1. Eventual obstructions (mud, debris, grass) of ditches, pits, pipes and discharging structures shall be removed by a periodic cleaning.
7	Transformers oil/water system	See detailed description at next paragraph TRANSFORMER OIL WATER RECOLLECTION SYSTEM DESCRIPTION	5 years Always after every firefigthing event.	See detailed description provided in next paragraph TRANSFORMER OIL WATER RECOLLECTION SYSTEM DESCRIPTION
8	Sewage system	Sewage treatment and septic tank. In the bottom of the septic tank, the solid particles will sedimentate. When the level of the solids reaches half height of the tank, typically the solids must be pumped out. Following the first emptying, it is possible to define better the required emptying time, according to the supplier operation manual. Periodical visual inspection of the tank level is therefore required.	In the first year: every three months. After the first year, to be defined according to the results of the first inspections	Pumping out of the solids by means of portable pump.
9	Power House slopes protection devices	Visual check of: status of barriers and rock-fall nets and protection barriers (if present) foreseen on benches above the yard; accumulation of debris behind protection net; potential instable wedges. Possible crack opening on the shotcrete or rock of the exposed front of excavation. Shotcrete and bolts where foreseen. Drainage water percolation.	1 year Always after very heavy rainfall event occurred.	Possible problems observed shall be measured, documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by photos. Than appropriate actions taken (see NOTES). In case of corrosion or damage to steel parts (ropes, pillars, junctions, meshes,..) painting against corrosion shall be done, if possible, or alternatively the part shall be substituted. In any case all anomalies shall be recorded and appropriate actions taken (see Note). Excessive accumulation of debris behind protection net (if present) shall be removed by skilled people. Scaling of instable wedges shall be done by skilled people. Extended areas of damaged shotcrete shall be cleaned and repaired. Damaged bolts shall be replaced.

				In case of continuous flow of water is observed coming out from drainage holes, the flow shall be monitored (quantifying the flow and reporting it on a spreadsheet). Than a specialist designer shall be consulted. Ordinary maintenance of all steel and concrete parts shall be done, as needed.
10	Submergible parts of the structure	In case of flood event all submerged structures shall be than inspected and visual check of all part shall be made.	Always after flood event	Ordinary cleaning and drying of all floors below level of the river, inside Power House, taking water for cleaning from nearest available water taps.
11	400kV line anchorage	Visual check of status of 400kV line anchors and anchorage devices behind Power House	2 years	In case of corrosion or damage to steel parts (ropes, rings, junctions) painting against corrosion shall be done, if possible, or alternatively the part shall be substituted. In any case all anomalies or movements shall be recorded and appropriate actions taken (see Note). Ordinary maintenance of all steel parts shall be done, as needed.
12	Yard and access roads	Visual check of status of the access road and relevant cut or embankments. Visual check of status of the yard and relevant works (gates, nets, protection walls, barriers, architectural elements)	Every year, preferably after rainy season Always after very heavy rainfall or flood or earthquake event occurred.	In case of major instability phenomena are observed, they shall be monitored, documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see Note). Along the road eventual obstructions of ditches, pits, pipes shall be removed by a periodic cleaning. On roads and yards ordinary maintenance of all steel and concrete parts (gates, nets, protection walls, barriers) shall be done, as needed, and repaired if and when necessary.
13	Tailrace Hole	It is requested, in dry season, the visual check of status of the tailrace hole (for the portion outside the water) and relevant sustaining structures (walls, supports, etc.). It is recommended a check of the bathymetry along the tailrace hole in front of the draft tube. This check is to be done preferably in dry season and preferably in the moment of turbines stop.	First time 1 wet season after Plant commissioning, than every 2 years. After 5 years, every five years. Always after very heavy rainfall or flood event occurred.	In case of major instability phenomena are observed, they shall be monitored, documented and reported in all details, where possible quantifying the extension of the damage in a dedicated spreadsheet, in relevant drawings and by a set of photos. Than appropriate actions taken (see Note). Damages to the protections shall be repaired as soon as possible. The bathymetry shall be compared with the tailrace hole geometry indicated in design drawings and in as built drawings. In case of significant (more than 1m in elevation) end extended differences in respect to the design profile, or in case of obstruction of the draft tube outlet, the actual profile shall be duly recorded and reported, quantifying the extension of the damage in

				relevant drawings and by a set of photos if possible. Than appropriate actions taken (see Notes).

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 21 - Principal aspects and actions required for the Power House maintenance plan

D.3.11SWITCHYARD MAINTENANCE

What follows is referred to one yard but applies for both left and right Power Plants.

The general rules for maintenance needed for the CIVIL WORKS at Switchyard are resumed in the following table.

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported again in the following table.

Operation and maintenance of electromechanical equipment and of all towers are described in relevant EM equipment operation manuals.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Concrete structures	Visual control and inspection of exposed concrete surfaces, relevant to: • EM equipment foundation blocks, • Cable trenches • control building • Water tank • Drain system structures (pits, ditches, dissipating structures)	Every year	See NOTE 3). The first prescription about civil work regards the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, for which it shall be applied the prescriptions that are provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter 0 " DAM CIVIL WORKS", to which reference shall be made (they are not reported again here).

2	Control building and switchyard area finishing and architecture	Visual inspection of all the architectural and finishing elements of the building and of the yard, such as: Fence, illumination posts, illumination system and lamps, windows, doors, pavements, paintings, access roads, external yard finishing, pedestrian walks, gutters, guardian box, fountain. The inspection shall be carried out by skilled people, able to identify possible defects that may require ordinary or extraordinary maintenance works.	First time, 1 year after impounding, then every 5 years	See NOTE 3). Possible wearing out, damages or defects on architectural elements shall be recorded and appropriate actions prospected to the Plant Owner for his action (see Note). Ordinary maintenance of fence, roads, illumination system and ancillary works shall be done, as needed.
3	Sewage system	Sewage treatment is made in the septic tank, located near the control building. In the bottom of the septic tank, the solid particles will sedimentate. When the level of the solids reaches half height of the tank, the solids must be pumped out. Following the first emptying, it is possible to define better the required emptying time, according to the supplier operation manual. Periodical visual inspection of the tank level is therefore required.	In the first year: every three months. After the first year, to be defined according to the results of the first inspections.	Pumping out of the solids by means of portable pump.
4	Control building water system	Visual inspection of all the water system (tanks, pipes, pipelines, valves, roof gutters, taps and all fittings) inside and outside the Switchyard building. Water system is described in relevant report. Test of functioning of all pumps located in the lower tank chamber. Check of potabilizer. Check of the air chamber of the pump inside the tank, according to the operation manual of the pump supplier. All these checks shall be of visual type, made by a qualified plumber.	1 year, preferably after the rainy season Differently only in case specific defects or leaks on the hydraulic system are observed.	Possible filtration, damages or cracks on tank structure shall be recorded and appropriate actions taken (see Note and see operation 1 above). Exposed reinforcement bars shall be treated as indicated at point 1. On off and functioning tests on all pumps, according to the relevant supplier operation and maintenance manual. Every year lamps of UV potabilizer device (if present) and filters must be changed. Ordinary maintenance and cleaning of pipes, pipelines, valves, roof gutters, taps and all fittings to be done periodically, as needed.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 22 - Principal aspects and actions required for the Switchyard maintenance plan

D.3.12ACCESS ROADS MAINTENANCE

Here below are resumed the main guidelines and actions required for the maintenance plan relevant to the ACCESS ROADS pertaining to the site of Batoka.

For the main (asphalted or paved) roads reaching the site, the maintenance and operation activities shall be undertaken by the national roads authorities (by Zambia and Zimbabwe, as far as their territory of competence) in the frame of their regulations and procedures.

Pr	Civil Feature	Description of check of performance indicators	frequency	Maintenance Actions - Remarks
1	Site access roads stability	Visual check of: • status of shotcrete, bolts, drainages and drain ditches foreseen on benches • potential instable wedges. • Signal of movement or slides of slopes. • Debris accumulation on benches or drain ditches. • Check of possible crack openings or fissures or deterioration of concrete surfaces. In case of continuous flow of water is observed coming out from drainage holes, the flow shall be monitored (quantifying the flow and reporting it on a spreadsheet).	Yearly, preferably at the end of rainy season. Additionally in case of an exceptional heavy rain or earthquake event.	Any anomaly shall be duly recorded and signalled. Deeper checks (and check of possible dependence from meteo conditions) to be prescribed on case by case basis. If sliding phenomena or cracks opening are observed, they shall be monitored and, if necessary, slope stabilization measures reconsidered (see NOTES). In case of instable wedges to be scaled, the scaling shall be done by skilled people. Extended areas of damaged shotcrete shall be cleaned and repaired. Damaged bolts shall be replaced. Accumulation of debris on roads or accessible benches or any obstruction of drain ditches or pipes shall be removed. Than a specialist designer shall be consulted. Ordinary maintenance and cleaning of the drain ditches, as needed.
3	Kerb, parapets, masonry protection walls, retaining walls, concrete paving or ramps	Visual check of good status of the concrete surface, including check of: • Presence of fissures or cracks; • Detachment of finishing concrete layers • Damages (erosion, holes, cracks) on concrete surfaces	In case of Power Waterways dewatering.	See NOTE 3). Any anomaly shall be duly recorded and signalled, possibly associated with measures of meteo conditions (see NOTES). Repairing intervention, if needed shall be properly designed and conducted by skilled people, in safe conditions.

NOTES:
1) In case of results or controls evidenced anomalies or problems, the measures shall be repeated for check, and then, if confirmed, the Plant Management direction shall be immediately informed and Direction involved in the dam safety consulted. According to the case the designer or a specialist consultant shall be consulted.
2) All operations shall be done by skilled personnel, instructed and authorised by the Plant owner for the operation required.
3) For check of concrete surfaces and of the status of the building and their finishing, the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing" in table reported in chapter D.3.2 shall be applied.

Table 23 - Principal aspects and actions required for the Access Roads maintenance plan

The ordinary frequent maintenance operation is essential to maintain functional the access to the site.

This continuous maintenance operation shall be not neglected or considered as secondary, to maintain efficient the roads, and resources and planning for such activity shall be organized by the PMS.

D.3.13PERMANENT CAMP

What follows is referred to one camp but applies for both left and right Power Plants.

For the PERMANENT CAMPS maintenance, the following general instructions apply.

As far as the prescriptions for the civil works regarding the check of concrete surfaces, of the reinforced concrete structures and of the status of the building and their finishing, reference should be made to the prescriptions provided in Pr.6 "Concrete surfaces" and Pr.7 "Control building, access buildings civil works and dam crest finishing " in table reported in chapter D.3.2, which are not reported here again.

General prescriptions for roads maintenance described in previous chapter D.3.12 "ACCESS ROADS" are valid.

General prescriptions for maintenance of excavation fronts and local supports of excavation and backfilled areas as described at point 9 of table reported in chapter D.3.10 are valid.

The size of the camp, its facilities and schemes of its services (water and power supply) are described in relevant drawings of Feasibility Design, and are assumed will be detailed in further steps of the design.

For the detailed instructions of operation and maintenance of the houses, the services systems (water, sewage, electric, ventilations or air conditioning, fire-fighting systems) pertaining to the houses and to the common spaces (canteen, pool, guardian room, etc.) dedicated specific manuals or instructions will be provided out of this report for the perusal of the staff charged to conduct and maintain the camp, when the detailed design will be produced.

It remains a key and essential factor for the proper use and long life of the permanent camp, the ordinary frequent maintenance operation of indoor and outdoor spaces (included the green areas).

This continuous maintenance and ordinary curing operation shall be not neglected or considered as secondary, to maintain efficient the camp, and resources and planning for such activity shall be organized by the PMS.

D.3.14HYDRAULIC DEVICES AND MAIN CONTROL EQUIPMENT MAINTENANCE

GENERAL

The operations and maintenance manuals relevant to the Electro mechanical (EM) and Hydraulic steel structure (HSS) equipment will be provided by the EM and HSS equipment suppliers.

Mechanical and electrical equipment require appropriate maintenance and testing. Gates, lifting equipment and power supplies should be continuously maintained during frequent inspections and minor maintenance works.
The ultimate aim of the maintenance activities is to ensure that the plant is capable of reliably performing its operational functions with no forced outages and at the minimum maintenance cost. The equipment must always be in good working order and be capable of both normal and emergency operation.

In this chapter guidelines for mechanical and electrical equipment main testing and maintenance operation are provided because they are of utmost importance being most of these equipments essential for the safe operation of the plant.

The maintenance of all such equipment shall be carried out in any case according to the detailed instruction provided in their relevant manuals that will prevail in case of conflict with the general instructions here reported.

What follows can be adjusted or modified or updated as needed with the development of the detailed design and of the construction.

TIME BASED TESTING, INSPECTION AND MAINTENANCE

Here below there is provided a table indicating the type and frequency of tests that are envisaged to be conducted for all the important equipment of the plant, in order to be sure that they function properly when their use is needed.
It is essential to document thoroughly the measured results and to identify any declining trend of any component of the plant.

The guidelines provided in the table are grouped where possible for the following Type of Equipment:

A) EMERGENCY POWER SUPPLY SYSTEM
   - ✓ Diesel generators for Emergency Power Supply

B) HYDRAULIC GATES
   - ✓ Spillway radial gates (with oleo-dynamic unit)
   - ✓ Middle Outlet downstream gates (with oleo-dynamic unit)
   - ✓ Power House draft tube gates (with oleo-dynamic unit)
   - ✓ Middle Outlet upstream gates

- ✓ Power Tunnel upstream gates

C) STOPLOGS

- ✓ Spillway stoplogs
- ✓ Power House draft tube stoplogs

D) VALVES

- ✓ Power House Main Inlet Valves
- ✓ Power House ecological discharge pipe guard valves
- ✓ Power House ecological discharge valve
- ✓ Power House hydraulic system pipes and tanks valves
- ✓ Transformers oil-water discharge system pipes and tanks valves

E) CRANES

- ✓ Dam crest crane (for Spillway and Middle Outlet upstream gate)
- ✓ Power Waterways Intake Gates monorail crane
- ✓ Power House main crane
- ✓ Power House auxiliary crane
- ✓ Power House Erection Bay gantry crane
- ✓ Power House stoplogs gantry crane
- ✓ Power House internal monorail crane (for draft tube gates lifting)

F) HOISTS

- ✓ Power House drain system pumps hoists
- ✓ Power House diesel generator building hoist (for transformers)
- ✓ Power Waterways Intake Gates hoists
- ✓ Power Waterways Intake Gates control room hoist (for pumps)
- ✓ Dam Pumping system pits hoists
- ✓ Dam diesel generator building hoist (for transformers)
- ✓ Switchyard control building hoist (for transformers)
- ✓ Penstocks man-hole chambers hoist

G) LIFTS

- ✓ Power House lifts
- ✓ Dam cable duct lift

H) PUMPS

- ✓ Dam drainage pumping system
- ✓ Power House drainage pumping system
- ✓ Power House fire fighting and service water system pumps

HYDRAULIC DEVICES AND MAIN CONTROL EQUIPMENT MAINTENANCE				
Maintenance Category	Type of Equipment	Check Activity	Frequency (*)	Description of the tasks
Testing (*)	EMERGENCY POWER SUPPLY SYSTEM	Start, Load, Stop	Monthly	Verify start reliability of diesel generator on loss of normal power supply and lubrication oil pressure, generating set vibration, fire valve status. Ensure batteries are fully charged. After use check and refill if necessary gasoil in the tanks.
	HYDRAULIC GATES (with their Hoisting devices)	Opening/ Closing fully Partial in case of drought	Yearly	Test operate gates and record motor currents, gate positions and time during gate operation. Use normal and emergency power supply. Record power absorbed by relevant motor and lifting/closing times. Check gate and valve operation throughout the full gate and valve opening and closing stroke. If necessary gate position re-calibration and testing. Check for foreign noises and smells from motors and control equipment.
	HYDRAULIC GATES (with their Hoisting devices)	Non-destructive testing of gate hoists	5 yearly	Testing of ropes, equipment attachment points
Testing (continue)	STOPLOGS	Opening/ Closing fully	2 yearly	Test operate stoplogs, gate positions and time for their installation. Use normal and emergency power supply for their lifting. Record power absorbed by electric motor and lifting/closing times.
	VALVES	Opening/ Closing fully Partial in case of drought	Yearly	Use normal and emergency power supply. Record pressure of hydraulic servomotors, or power absorbed by electric motors and opening/closing times. Check setting of hydraulic set relief valve. Check possible leakage of water through the closed valve.

	CRANES and LIFTS	Non-destructive testing	5 yearly	Testing of ropes, equipment attachment points. Test the movement of the main crane and its hoist for all possible configuration and extension, with and without applied loads. Test operate crane or lifts and record motor currents, positions and time during operation. Use normal and emergency power supply. Record power absorbed by relevant motor and lifting/closing times. If necessary crane rail alignment re-calibration and testing. Check for foreign noises and smells from motors and control equipment.
	HOISTS	Non-destructive testing	5 yearly	Testing of ropes, equipment attachment points. Test the movement of the monorail crane and its hoist for all possible configuration and extension, with and without applied maximum nominal load.
	PUMPS	Start, Load, Stop	2 monthly	Verify start reliability and functionality for the set levels of functioning, check proper functioning in relation of relevant control and floating system in the pits.
	General (as far as applicable to all type of equipment)	Opening/ Closing fully	Yearly	Inspect controls indications, sirens and warning lights as applicable. Water level sensor re-calibration and testing.
Inspection	General (as far as applicable to all type of equipment)	Visual inspections and Checks	Monthly	Check for any anomalies, hazards or security risks. Check all lubrication and top-up lubricants as required. Check gates and valves hydraulic system oil levels.
	EMERGENCY POWER SUPPLY	Close examination of diesel generator set conditions, replace air and oil filters	Six monthly	Verify coolant, motor heating, fuel supply, battery charge. Check tension of driving belts. Check electrical auxiliaries. Check all alarms and shutdown switches.
	HYDRAULIC GATES (with their Hoisting devices)	Visual examination of gates, seals, guide pads, steel ropes, screws, winch, brakes, contactors.	Six monthly	Check any wear and tear; measure vibration, motor electric insulation resistance. Examine ropes for broken strands, crushing, kinking. Examine gear train. Inspect paint and check any corrosion. Check gearbox oil level. Check brakes lining. Check gate lifting limit switches.

	VALVES and their Actuators	Visual examination of valve seal, sliding guides, leakage.	Six monthly	Check wear and tear, paint, and water leakage. Check hydraulic set leakages and re-pumping time. Verify accuracy of their position indication instruments.
	CRANES	Visual examination of wheels, guide pads, steel ropes, screws, winch, brakes, contactors.	Six monthly	Verify mechanisms, check tension of driving belts. Check electrical auxiliaries. Check brakes and control devices. Check all alarms and shutdown switches.
	STOPLOGS	Visual examination of seals and sliding guides	5 yearly	Check wear and tear and paint. Check status of sealing elements.
	PUMPS	Visual examination of pumps and relevant guides and pipes	5 yearly	Check wear and tear and paint. Check status of sealing elements, check status of anchorage and bolts.
Maintenance	EMERGENCY POWER SUPPLY SYSTEM (**), GATES, VALVES, CRANES, HOISTS, LIFTS, PUMPS	General maintenance	Monthly/ Annually	As per supplier's O&M recommendations (Grease bearings, check for hydraulic system oil leaks, motor contactors, brake solenoid, brake pads, gate seals, gate guide pads and all other recommended tasks). Re-order any spares. (**)
	CRANES, HOISTS and LIFTS	Check lubrication of moving parts of hoists and winch units	Monthly	Grease moving parts as required. Other actions as per supplier's O&M recommendations
	General (as far as applicable)	General safety checks	5 yearly	Check condition of all handrails, ladders and other equipment critical for personnel safety
	General	Updating of documentation	Annually	Ensure operating procedures, emergency planning and contact documentation and other relevant reference documentation is up to date.

Table 24 - Hydraulic devices and main control equipment maintenance

(*) At Batoka it is likely that some valves or gates or cranes will be frequently operated under flow, in which case the corresponding formal tests may not be required. However, it needs to be ensured that the observation and data recording requirements described in the table above are met, and that at least some of these valve operations are carried out via the emergency diesel power supply.

(**) Maintenance tasks and frequency for the emergency diesel generator should be based on the diesel generator supplier's O&M manuals. As a minimum the following checks and appropriate follow-up actions would be expected:

- Walk around visual inspection;
- Check engine oil and coolant levels;
- Check fuel level;
- Check fan belt tensions;
- Check hoses for loose connections or deterioration;
- Check battery connections for corrosion and check battery electrolyte;
- Check dust and oil build-up, oil leaks and fuel leaks;
- Check and empty bunding;
- Check all system protection by simulating a fault;
- Check all battery caps as applicable;
- Tighten all electrical connections and exhaust connections;
- Change oil and filters;
- Start the diesel motor and check gauges and meters are working correctly;
- Run on load bank for 30 minutes.

GENERAL GUIDELINES FOR EQUIPMENT CONDITION MONITORING

Condition assessment of equipment and condition-based maintenance are the two fundamental aspects of predictive maintenance which is aimed at carrying out maintenance activities only when the decline of equipment performance reaches a predefined level, thereby minimizing both the predictive maintenance and the occurrence of equipment malfunction.

Condition monitoring and condition-based maintenance guidelines are described below.

- TESTING OF ELECTRIC MOTORS AND DATA LOGGING

  The following data should be recorded during all testing of electric motors:
  - Date and time;
  - Name of operator carrying out the test;
  - Gate position;
  - Motor current and voltage.

  All test data should be analysed by suitably qualified personnel and compared against previous results to check for any changes in performance. The results shall be reported appropriately and any necessary corrective maintenance actions shall be undertaken.

- INSPECTION AND DATA-LOGGING OF GATE, VALVE AND LIFTING EQUIPMENT OPERATION

  It is essential that from time to time gate, penstock inlet valve and lifting equipment operation is recorded and observed by suitably qualified and experienced personnel. In addition to an assessment of gate and valve opening and closing times, the observations should include gate sealing and seating characteristics, gate vibration, noise or any other signs of distress.

- INSPECTION OF COATING SYSTEMS

  The life of hydraulic gates, bulkheads and associated lifting equipment can be extended significantly if their coating systems are monitored and maintained in good condition. Generally coating systems for such applications have a life expectancy of 20 to 40 years, depending on coating technique and quality of the paint system. In addition, life expectancy largely depends on environmental factors and the life may be significantly reduced in presence of sand erosion that cannot be excluded at Batoka Dam site. Frequent repairs to the coating systems are likely to be required to prevent corrosion from affecting the structural integrity of the gates.

- MAJOR REFURBISHMENT OR REPLACEMENT

  End-of-life is considered to be the point at which either major component replacement or refurbishment is required. Typically gate refurbishment will involve gate removal, sand-blasting and re-painting, seal replacement, repair or replacement of gate rollers and/or gate roller bushes and repair work to the embedded steel work in the gate slot. Repair or replacement of components of the lifting system will be carried out as required at the same time.

D.4 TYPICAL FORMAT FOR INSPECTION CHECK SHEET

Hereinafter is provided the typical format for inspection check sheet.

BATOKA PLANT - ISPECTION CHECKLIST

Inspection	Comment
Inspector (name)	
Date and Timing	Date: Inspection start time: Inspection finish time:
Reservoir (or river (1)) Level	_____m a.s.l.
Flow discharged (1)	_____m^3/s
Conditions	Sunny / Overcast / Raining / Windy / Storm / Flood / Earthquake
Rainfall over past week	High / medium / low _____mm (if available)

(1) = only if applicable, depending on where the inspection is conducted.

Checklist n. / (a)	WORK: (b)					
Operation /Civil Feature	Aspect inspected / Performance Indicator	Observed		Significance (f)		
		Yes	No	Minor	Moderate	High
(c)	(d)	(e)		(f)		
Comments	(e)					
(c)	(d)	(e)		(f)		
Comments	(e)					

(a) = insert the progressive number and date
(b) = insert the part of the work checked (for instance "BRIDGE")
(c) = insert the civil feature/operation, taking the description from the table reported in this report at the chapter corresponding to the work under check (for instance "Bearings periodic check", or adding other description as far as needed or appropriate.
(d) = insert the description of the performance indicator or the aspect inspected (one per line), taking the description from the corresponding column in the table reported in this report at the chapter corresponding to the work under check (for instance "Absence of movement or anomalous deformations"), or adding other description as far as needed or appropriate.
(e) = insert the observation of presence or not of the performance indicator (Place a ✓ in the appropriate box), and/or if necessary qualifying the observation in the box of the comments. Add if necessary comments on any abnormal or changed conditions.
(f) = qualify the performance indicator as follows:

Minor = The performance indicator observed but does not require maintenance.
Moderate = The performance indicator observed and requires action to remediate.
High = Prompt action should be taken to address the issue

Signature: _____ Date:_____

(Inspector)

Other lines can be added to this format as far as needed.

PART E - EMERGENCY PREPAREDNESS PLAN (Framework Plan)

E.1 INTRODUCTION

E.1.1 CONTENT AND STRUCTURE OF THIS PART

This is the PART E **"EMERGENCY PREPAREDNESS PLAN** (Framework Plan)**"** of the Batoka Dam Safety Plan. This part contains the Batoka Emergency Preparedness Plan (Framework Plan) that includes:

- description of types of emergencies and how to identify them,
- actions to take in an emergency.
- preparedness and Emergency Response
- Dam Break analysis

Based on this framework, the Emergency Preparedness Plan will be prepared during implementation of the project, in compliance with WB guidelines (OP 4.37), not later than one year before the initial filling of the reservoir.

This part of the report is divided in the following chapters:

1) FOREWORD
   Describes the content of the report and its structure.

2) STRUCTURE AND REVIEW OF THE PLAN
   It outlines the structure and the purpose of the plan, the needs for its review and for its implementation in details, and the principles on which is based.

3) EMERGENCY CASES
   This chapter provides a synthetic description of the type of emergencies.

4) INITIATION OF THE EMERGENCY ACTION PLAN
   This chapter describes how to identify and evaluate an emergency case.

5) EMERGENCY RESPONSE AND ACTION PLANS
   This chapter provides a description of the initial response and emergency inspections to be carried out in case of need, and the typical emergency action plans identified for this project.

6) EMERGENCY PREPAREDNESS PLAN
   This chapter provides the plan and procedure for emergency preparedness.

7) CONTINGENCY PLAN

This chapter illustrates how to manage exceptional cases having impact on the safety of the people and of the structures, and relevant alarms triggering.

8) DAM BREAK ANALYSIS

This chapter illustrates the results of the hydraulic calculation to identify the flooded areas in case of dam break. The evaluated scenario foresees the incoming of an extreme climate event meteorological event, which causes dam overtopping and presents the study of the flood propagation for a river stretch of approximately 120 km length, along the Zambezi river, from the Batoka dam up to the Lake Kariba.

.

E.2 STRUCTURE AND REVIEW OF THE PLAN

E.2.1 GENERAL

This Emergency Preparedness Plan (Framework Plan) for Batoka Plant has the purpose to limit possible damage to the dam and appurtenant structures and also areas downstream, and prevent loss of life by guiding personnel on what to do, when and how.

This document sets out the guidelines to:

- define, identify and evaluate events with the potential to compromise the dam and appurtenant structure safety.
- Establish procedures for declaring an event as a dam safety emergency.
- Detail actions to be taken in response to the dam safety emergency (through Emergency Action Plans (EAP).
- Establish communications to minimise the consequences of the dam safety emergency.

It shall be integrated and detailed by the PMS that will be in charge to organize and implement it.

This document presents procedures for Emergency Action Plans (EAP) for the dam site.
Response plans for areas outside Batoka site will be developed separately by the Local Authorities.

This plan must be completed to reflect final design, actual conditions on site, and afterwards shall be reviewed and updated:

- on completion of the Plant Construction
- During operation:
    - When any significant change to the scheme occurs, including any changes to the operating rules of the Plant.
    - At intervals of 5 years.
    - Following any ownership change.

E.2.2 PRINCIPLES

The following principles underlie the emergency actions for all dam safety emergencies:

- PERSONNEL SAFETY must be considered FIRST at all times;

- The primary defence if a dam failure scenario is developing or if the dam is seriously damaged, is to LOWER THE RESERVOIR LEVEL. This is achieved by opening outflow controls;

- If a serious leak occurs in the Power Waterways or Power House, or they are seriously damaged, then CLOSE THE POWER WATERWAYS GATES to stop the water source.

This EPP assumes that the Plant is manned full time, 24 hours a day, 7 day a week, and that the dam site is occupied and managed by a full time PMS Operator working and connected with the Owner(s) of the Plant.

It is assumed that for the life of the dam the PMS/Owner(s) is responsible for ensuring that appropriate measures are taken and sufficient resources provided for the safety of the dam.

E.3 EMERGENCY CASES

E.3.1 TYPE OF EMERGENCIES and RESPONSE LEVEL MATRIX

A Dam Safety Emergency is an event that has the potential of endangering the integrity of the dam or appurtenant structures, and therefore requires immediate action.

The hazard events identified at Batoka Plant and the associated response levels are described in the following table that shall be used to initiate emergency action plan.

Some further clarifications and descriptions are given in next paragraphs, in sake of completeness.

RESPONSE LEVEL	TYPE OF EVENT								
	1a	**1b**	**2**	**3**	**4**	**5**	**6**	**7**	**8**
	Excessive leakage through dam	**Excessive uplift pressures or excessive seepage/piping through dam foundations**	**Excessive leakage through power waterways or abutments**	**Earthquake**	**Structural failure of devices for water control (Middle Level Outlet, Spillway, Power Waterways, Power Houses, River diversion plug)**	**Gate failure/ power loss/ equipment failure/ fire**	**Excessive plunge pool scouring endangering dam foundations**	**Extreme weather warning**	**Sabotage/ accident**
Internal Alert Situation can be managed internally. Outside notification NOT required	Excessive leaks through dam. Movement of a portion of dam body along horizontal RCC surface detected through monitoring of pendula, deformometers or joint meters. Dam drains and/or piezometers or displacements monitoring passes from "routine" to "alert" criteria.	Excessive leaks recorded through dam drains intercepting foundations. Movement or displacements of a portion of dam body detected through monitoring of pendula, deformometers or joint meters. Ground aquifer level and/or flow trend start to raise independently from reservoir fluctuations. Dam foundation drains and/or piezometers monitoring passes from "routine" to "alert" criteria.	Excessive leaks through power waterways drainage system. Dam drains and/or piezometers or displacements monitoring passes from "routine" to "alert" criteria.	Earthquake less than OBE recorded through dedicated instruments.	Excessive leaks detected through available drains (MLO drains, Spillway external drains, Power Waterways drain system, Power Houses drain pumping system). Local drains monitoring system passing from "routine" to "alert" conditions.	Failure of spillway gates or MLO gates, or Power Waterways gates or Power House gates, or relevant power/control systems.	Anomalous scouring recorded through specific bathimetric check after exceptional flood event or after exceptionally prolonged use of spillway/middle Level outlets. Movement or displacements of a portion of dam body detected through monitoring of pendula, deformometers or joint meters. Dam foundation drains and/or piezometers monitoring passes from "routine" to "alert" criteria.	Heavy rain forcast. Reservoir level recrods above Maximum Operating Level. Reservoir levels monitoring passes from "routine" to "alert" criteria.	Any incident of threat of attack
Response Level I Conditions might lead to downstream flooding. Outside notification required	Increasing rate of excessive leaks. Increasing rate of dam displacements or movements. Visible crakcs in dam galleries or dam body.	Increasing rate of excessive leaks through foundation drains. Heavy seepage carrying fines detected through piezometers and dam foundation drains. Ground aquifer level and/or flow trend raising independently from reservoir fluctuations. Visible movements or cracks in the dam body. Anomalous displacements recorded through invert pendula or dam deformometers.	Increasing rate of excessive leaks through left abutment drain system. Seepage carrying fines detected through left abutment piezometers and drains. Groundwater level and/or drain flow trend raising independently from reservoir fluctuations. Visible incipient movements or cracks development in the dam abutments. Abrupt change in Power House(s) generating units performace independently from the operator of the plant (like abrout reduction of turbined flow, turbine stop, energy production abrupt decrease)	Earthquake greater than OBE recorded through dedicated instruments. Felt by all. People and animals are alarmed, and many run outside. Furniture moves, and objects fall from walls and shelves. Damage to buildings.	Increasing rate of excessive leaks through local drain system. Seepage carrying fines detected through drains. Visible incipient movements or cracks development in visible portions of the works (Visible signs of structural failures of Intake Tower, or Middle Level Outlet, or gate shaft, surge shaft, spillway piers or chute). Abrupt change in hydraulic device performace independently from the operator of the plant (like abrout reduction or variation of discharged flow) Excessive leaks through river diversion tunnels (through visual detection, if possible, considering they will be often submerged).	Spillway or MLO or Power Waterways gates unable to work in conjunction with possible flood events. Reservoir level trend raising above the maximum Operating Level.	Scouring in the plunge pool attaining dam downstream toe exceeding depth corresponding to dam foundation bottom gallery level (not instrument available, specific monitoring check to be conducted if necessary). Visible movements or cracks in the dam body. Anomalous or abrupt displacements recorded through invert pendula or dam deformometers.	Heavy rain forcast. Reservoir level records above Maximum Operating Level, but below dam crest level. Reservoir level trend raising. Spillway and Middle Level Outlets already fully opened or out of operation.	Attack that threatens the integrity of the dam or spillway or Power Waterways
Response Level II Prepare for evacuation	Drained flow in lower galleries approaches the pumps capacity limits . Leaks appears on dam downstream face through RCC horizontal lifts joints. Uncontrolled movements of portions of dam	Dam foundation subject to uncontrolled seepage or piping or displacements along main joints of the rock develops a major uncontrolled outflow or source of evident movements of the dam (or of a portion of it)	Abutment(s) rock subject to uncontrolled leaks, developing major uncontrolled outflow or source of evident movements of the rock wedges, power tunnel lining or dam body (or of a portion of it)	Instability of dam resulting from earthquake (centimetric abrupt displacements or movements recorded through pendula, deformometers and joint meters. Visible Cracks on dam body, strong increase of leaks through dam drains)	Left or right abutments subject to uncontrolled leaks, developing major uncontrolled outflow or source of evident movements of the rock wedges, power tunnels linings or dam body (or of a portion of it). Middle Level Outlet structures subject to uncontrolled leaks or movements. Spillway structure subject to uncontrolled leaks or movements affecting the dam crest and/or its retention capacity.Reservoir level trend raising, predicted to exceed dam crest level.	Spillway or MLO or Power Waterways gates unable to work in conjunction with possible flood events. Reservoir level trend raising, predicted to exceed dam crest level.	Dam foundation at downstream toe subject to uncontrolled scouring or displacements along main joints of the rock developing a major uncontrolled source of evident movements of the dam (or of a portion of it)	Reservoir level trend raising, predicted to exceed dam crest level.	As above
Response Level III Evacuation is necessary	Dam collapse imminent	Dam collapse imminent	Dam or power waterways or abutments rock collapse imminent	Integrity of dam visibly jeopardized by earthquake	Dam or power waterways or abutments rock collapse imminent. River diversion plugs failure imminent. Dam overtopping imminent.	Dam overtopping imminent	Dam collapse imminent	Dam overtopping imminent	As above

Table 25 – Response Level matrix

E.3.2 TYPE OF EMERGENCIES

A Dam Safety Emergency is an event that has the potential of endangering the integrity of the dam or appurtenant structures, and therefore requires immediate action.

A national civil emergency is an event that has been declared either a state of local emergency or a state of national emergency by one of these institutions:

- ZRA,
- a Zambian Provincial Authority
- a Zambian Government Minister
- a Zimbabwean Provincial Authority
- a Zimbabwean Government Minister

A national civil emergency may occur due to an extreme flood which the Batoka Dam Project passes without incident.

There are substantially three categories of Dam Safety Emergencies:

- Internal Emergencies:
- Developing Emergencies:
- Imminent Emergencies.

Internal Emergency is when there is no danger of a dam failure, but flooding is expected to occur downstream. It can be dealt with internally by the operators of the PMS at Batoka Plant and no outside notifications are required.
The response level for this category of emergency is called "Internal Alert".
Developing Emergency is used when a potential dam failure situation is developing. It occurs only when there is some time still available for further analysis/decision s or corrective measures before uncontrolled release of water. The dam condition may be deteriorating but it is not judged likely to fail within hours. Notification to third parties is required.
The response level for this category of emergency is called "Response Level I".
Imminent Emergency is used when failure of the dam is considered imminent or has occurred.
It is when "time has run out" - a failure has either occurred is occurring or about to occur. This condition is declared when there is no longer any time available to attempt corrective measures to prevent failure. The dam has or is in the process of failing or is expected to fail within hours rather than days. Immediate notification to third parties is required.
Within this category of emergency falls the "Response Level II", for the evacuation preparation, and the "Response Level III" for evacuation.

The response levels for the hazard events identified (also described in next chapter) at Batoka Plant are described in the Response Level Matrix above.

E.4 INITIATION OF THE EMERGENCY ACTION PLANS

E.4.1 HAZARD CONTEXT

A dam hazard is an issue or event that has the potential to impact on the safety of the dam and the consequences downstream that the dam might influence. A hazard may be a natural event such as an earthquake, a structural problem, or an operational issue such as sudden changes of water discharge.

The hazard events relevant to Batoka are identified as follows:

- Important leakages through dam and its foundations
- Important leakages through power waterways
- Earthquake event
- Water control devices structural failure
- Gate malfunction, equipment failure, loss of power supply, fire
- Excessive plunge pool scouring endangering dam foundations
- Extreme weather warning (Rainfall / major flood event)
- sabotage

Events that would initiate the emergency action plan are summarised in *Table 25 – Response Level matrix*.

E.3.1 EMERGENCY IDENTIFICATION AND EVALUATION

Emergencies that may develop into a Dam Safety Emergency are by nature unpredictable.
This means that the response to incidents cannot be prescribed in detail. However, once an incident has occurred it can be assessed and actions taken to control and mitigate consequences by following pre-established procedures.

The initial notification of an incident that may develop into a Dam Safety Emergency may come from a number of sources:

- Regional event (e.g. earthquake, flood) recorded by national or local authorities,
- Observation by a Batoka Plant PMS staff member on site,
- Values read from monitoring site instruments by Batoka PMS staff.
- Observations by PMS operators.

In all cases actions should be based on:

1. Secure your own safety.
2. Alert others in the immediate area.

3. Notify the PMS and the Owner(s), if the case, and advise location, seriousness and nature of emergency event.

On receiving notification of an incident the PMS Operator assesses the seriousness of the incident. If the incident qualifies as an emergency, the PMS Operator will contact the PMS structure and the Owner(s) structure to cooperate in emergency management.

The PMS shall identify clearly the roles and responsibilities of those involved in emergency management. Some emergency events may require national government resources for assistance.
Contacts for notifications shall be reported in the formats provided at the end of this report, that shall be duly filled and always be updated by the PMS since the beginning of his work.

The PMS shall then develop, within its structure and the Plant Owner structure, specific flow charts for the notification process for the different cases of Imminent Emergencies notification and Developing Emergencies notification, as well as for the internal notifications applicable to an Internal Emergency.

In the redaction of such flow charts, the following general progressive steps (as far as necessary) shall be followed:

1) Check threshold for emergency: *Table 25*.
2) Document the emergency (circumstantiating at least the type and the location).
3) Notify to PMS and Owner(s) deputed structures. Only in the case of an internal emergency that are unlikely to have an impact on the downstream areas, the notification to the local authorities can be omitted.
4) The appropriate PMS personnel shall provide advice and support, and implement, if the case, the relevant Action Plan.
5) Consult external advice and support, if the case.
   Whenever the emergency implies evaluations and actions out of the technical, political or decisional competence of the PMS personnel, external advice is required. As far as technical or dam safety matters are concerned, consultancy of qualified engineering advice shall be seek.

As far as the particular case of the seismic event, it is here recalled that the structure is located in a seismic region.
The Seismic Hazard assessment at feasibility design level is presented in dedicated report "320 SEI R SP 001 C - Seismic Hazard Assessment, February 2016", to which reference is made, from which the following PGA values have been retained for the design:

-	0.23 g		SEE (Safety Evaluation Earthquake)
-	0.08 g	RP = 475	DEAS (Design Earthquake for Appurtenant Structures)
-	0.05 g	RP = 145 y	OBE (Operating Basis Earthquake)

This assessment is assumed will be refined during the development of detailed design.

E.5 EMERGENCY RESPONSE AND ACTION PLANS

E.5.1 INITIAL RESPONSE AND EMERGENCY INSPECTIONS

The initial response by the PMS or Owner(s) departments that will be deputed to receive the notifications of the PMS staff operating on site, in the case of a notification of an emergency event, is to assess and confirm the extent of the emergency event to enable an appropriate course of action to be implemented.

The initial response may be a decision to immediately begin lowering the reservoir level.

If failure of the dam is considered not imminent, then the initial response will involve checking operational conditions. The Director of PMS will make immediate arrangements for site inspections to be carried out.

The purpose of the Emergency Inspection is to:

- Detect physical indicators of failure modes (Imminent or Potential Dam Failure):
- Detect physical indicators limiting/preventing drawdown functionality and capability.

Emergency Inspections shall be undertaken by Surveillance Inspectors in accordance with the appropriate Emergency Inspection Checklists reported in the next paragraph. The checklists define the observations to be made and reported to the Director of the PMS structure. The list provided for the checklists is a reference format that can be integrated by the PMS staff according to the need.

The emergency inspection checklists do not mention the instrument monitoring, that is foreseen to be carried out as routine activity. The observations gathered by the instruments monitoring and provided in the foreseen inspection reports shall be combined and correlated with the visual observations and records gathered by the emergency inspection.

E.3.2 EMERGENCY INSPECTIONS CHECKLISTS

The following pages provide the inspection checklists of the main works of the Plant. They can be integrated as appropriate by the PMS.

The observations of this check shall be always correlated and integrated with the check and data available from the dam instruments.

Dam (Sheet 1 of 2)			
Emergency Inspection Checklist			
Go to Main Dam Left and Right Abutments and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Landslides in reservoir surrounds	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Dam upstream face	Damages or cracks in the visible part of dam u/s face	Y/N	
	Damages at Dam-rock contact	Y/N	
	Any right abutment damage, slides, sink holes, depressions, erosion etc.	Y/N	
	Any left abutment damage, slides, sink holes, depressions, erosion, etc	Y/N	
	Leaks inside dam galleries	Y/N	
	Leaks from Waterstop drains	Y/N	
	Is water line against upstream face a straight line?	Y/N	
Dam Crest	Any damage, cracks, depressions etc	Y/N	
Dam bottom	Leaks inside dam galleries and possible foundations seepage	Y/N	
Insert as far as of interest photographs of upstream area, right abutment and left abutment. The observations of this check shall be integrated with the check and data available from the dam instruments.			

Table 26 – EMERGENCY CHECKLIST 1 sheet 1 of 2 - Dam

Dam (Sheet 2 of 2)			
Emergency Inspection Checklist			
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Dam downstream face	Leaks through RCC	Y/N	
	Cracks on dam d/s face	Y/N	
	Damage or sign of movements or cracks at Dam- rock contact	Y/N	
	Any right abutment damage, slides, sink holes, depressions, erosion, water venues, signs of movements, etc.	Y/N	
	Any left abutment damage, slides, sink holes, depressions, erosion, water venues, signs of movements, etc.	Y/N	
	Seepage	Y/N	
Insert photo showing right abutment downstream detail	Insert photo showing downstream toe detail	Insert photo showing left abutment downstream detail	
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 27 – EMERGENCY CHECKLIST 1 sheet 2 of 2 - Dam

Spillway			
Emergency Inspection Checklist			
Go to Spillway and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Landslides in the reservoir	Y/N	
	Clogging of spillway gates by trunks	Y/N	
	Recent floods occurred	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Spillway gates	Visible damages or blockage of gate/s	Y/N	
	Any damage or leaks or wetting in the gates concrete piers	Y/N	
	Any visible movement of a gate concrete pier	Y/N	
	Leaks from gate itself or evident damages to the oleodynamic unit	Y/N	
Spillway chutes	Damages or cracks or cavitation in the visible part of concrete	Y/N	
	Aerators clogged or damaged	Y/N	
Spillway drains	Any unusual or important leakages	Y/N	
Insert as far as of interest photographs. The observations of this check shall be integrated with the check and data available from the dam instruments.			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 28 – EMERGENCY CHECKLIST 2 - Spillway

Power Waterways (applicable for each power waterway)			
Emergency Inspection Checklist			
Go to Power Waterways Drain Tunnels and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Landslides or instable wedges on left abutment downstream of dam	Y/N	
	Recent floods occurred	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Shotcrete Lining	Damages or cracks in the visible part of lining	Y/N	
	Any abutment damage, slides, sink holes, depressions, erosion, etc	Y/N	
	Leaks through fissures in rock or shotcrete (not from drain holes)	Y/N	
	Leaks from Watertight doors	Y/N	
Drains	Any unusual or important leakages	Y/N	
	Any drains obstruction or clogging	Y/N	
Insert as far as of interest photographs. The observations of this check shall be integrated with the check and data available from the dam instruments.			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 29 – EMERGENCY CHECKLIST 3 – Power Waterways

Gate Shafts Intakes (applicable for each power waterway)			
Emergency Inspection Checklist			
Go to Gates shafts and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Landslides or instable wedges on left abutment downstream of dam	Y/N	
	Recent floods occurred	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Gate shaft	Visible damages or cracks or movement of the visible part of lining, including upper room	Y/N	
	Any abutment damage, slides, sink holes, depressions, erosion, etc	Y/N	
	Leaks through fissures in the shaft	Y/N	
Gates	Visible damages or blockage of gate/s	Y/N	
	Any blockage or problem to lifting devices	Y/N	
Insert as far as of interest photographs.			
The observations of this check shall be integrated with the check and data available from the dam instruments			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 30 – EMERGENCY CHECKLIST 4 – Gate Shafts (Intake)

Surge Shafts (applicable for each power waterway)			
Emergency Inspection Checklist			
Go to Surge shafts and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Landslides or instable wedges on left abutment downstream of dam	Y/N	
	Recent floods occurred	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Surge shaft	Visible damages or cracks or movement of the visible part of lining, including upper structure	Y/N	
	Any abutment damage, slides, sink holes, depressions, erosion, instable wedges, leaks, etc	Y/N	
	Leaks through fissures in the shaft	Y/N	
	Any anomaly in functioning of turbines operation	Y/N	
Drains	Any unusual or important leakages through fissures in the surrounding rock or in drains	Y/N	
	Any drains obstruction or clogging	Y/N	
Insert as far as of interest photographs. The observations of this check shall be integrated with the check and data available from the dam instruments			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 31 – EMERGENCY CHECKLIST 5 – Surge Shafts

MIDDLE LEVEL OUTLETS and PLUNGE POOL			
Emergency Inspection Checklist			
Go to MLOs and PLUNGE POOL and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Recent floods occurred	Y/N	
Plunge Pool	Landslides or instable wedges on plunge pool abutments	Y/N	
Middle Level Outlets	Odd or exceptional MLO operation event	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Plunge Pool	Visible damages or cracks or movement of the visible part of left abutment deflecting wall.	Y/N	
	Any front of excavation visible damages, slides, sink holes, erosion, instable wedges, leaks, shotcrete cracks, etc	Y/N	
	Scoured material invading and obstructing the PH tailrace	Y/N	
MLO	Blockage or serious visible damage of: Upstream gate Downstream gate Steel lining Downstream concrete block structure Protection slab at dam downstream toe Control room	Y/N	
	Visible damages to Control units or control cables of hydraulic devices	Y/N	
Insert as far as of interest photographs. The observations of this check shall be integrated with the check and data available from the dam instruments.			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 32 – EMERGENCY CHECKLIST 6 – MLO and Plunge Pool

Power House (applicable to each power house)			
Emergency Inspection Checklist			
Go to Power House and compare condition with photographs			
Check		Change Yes/No	Comments
Reservoir	Reservoir level steady	Y/N	
	Unusual flow patterns	Y/N	
	Recent floods occurred	Y/N	
Power House	Landslides or instable wedges on power house excavation front	Y/N	
	Odd or exceptional units operation	Y/N	
Insert photograph of reservoir in normal condition			
Check		Change Yes/No	Comments
Power House	Visible damages or cracks or movement of the visible part of structure, including external buildings	Y/N	
	Any front of excavation damage, slides, sink holes, erosion, instable wedges, leaks, etc	Y/N	
	Leaks through PH structure concrete	Y/N	
Hydraulic or control devices	Blockage or serious damage of: Generation unit Transformers MIVs Ecological Valves Draft tube gates Drain pumps Control room	Y/N	
	Visible damages to OHL transmission lines	Y/N	
Insert as far as of interest photographs. The observations of this check shall be integrated with the check and data available from the dam instruments.			
Report to PMS general condition of dam and toe.			
Time and date of inspection:			
Inspector name:			
Received by PMS (date and time):		Operator Name:	
Notified to:	in date:	by:	

Table 33 – EMERGENCY CHECKLIST 7 – Power House

E.5.2 EMERGENCY ACTION PLANS

The following principles underlie the emergency actions for all dam safety emergencies.

- PERSONNEL SAFETY must be considered FIRST at all times.

- The primary defence if failure of the Batoka Dam is developing or if the dam is seriously damaged, is to LOWER THE RESERVOIR. This is achieved by means of Spillway and/or Middle Level Outlet devices, conjunctively with the Powerhouses units if available, as described in the next chapters of this part. The extent to which it should be lowered depends on the reason for the emergency. Advice should be taken from an experienced dam safety engineer in the Hydropower Development and Dam administration directorate.

- However, if the emergency is related to damage to the Power Waterways downstream of the Gate shafts, then this action should not be forcedly taken and the GATES SHOULD BE CLOSED in order to isolate the damaged area. The same in case of damage to Middle Level Outlets, relevant gates shall be closed to isolate the problem.

Emergency Action Plans shall be developed to cover the situations that could lead to the development of failure modes for the dam and the associated critical structures relevant to initiating events reported in table *Table 25* and recalled in the following table.

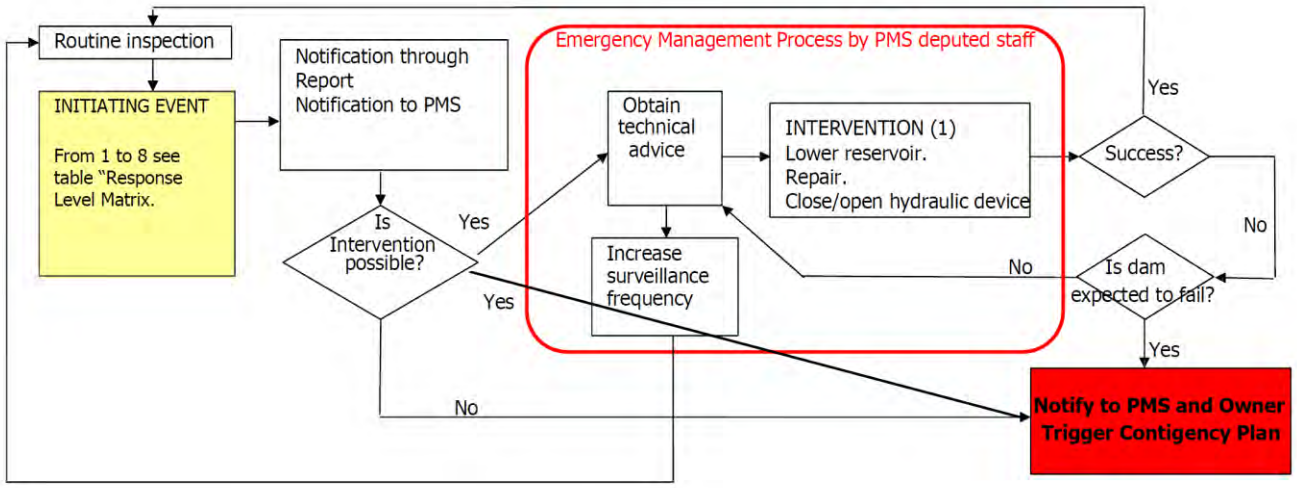
The plan shall be developed in order to foresee the implementation of a stepped notification procedure, following the above principles and the guidelines of table *Table 25*, to be applied on case by case basis to the emergency.

A typical Action Plan is reported in next figure. Emergency Action Plans have been developed to cover the situations that could lead to the development of failure modes for the dam and the associated critical structures diagrammatically one by one in the subsequent figures.

INITIATING EVENT (ref. table at par. E.3.1)	Failure Mode	Description of failure mode
1a Excessive leakage through dam	FM 1a	Drained flow in lower galleries approaches the pumps capacity limits. Leaks appears on dam downstream face through RCC horizontal lifts joints. Uncontrolled movements of portions of dam
1b Excessive uplift pressures or excessive seepage/piping through dam foundations	FM 1b	Dam foundation subject to uncontrolled seepage or piping or displacements along main joints of the rock develops a major uncontrolled outflow or source of evident movements of the dam (or of a portion of it)
2 Excessive leakage through power waterways or left or right abutment	FM 2	Leaks of Power Waterways Left abutment subject to uncontrolled leaks, developing major uncontrolled outflow or source of evident movements of the rock wedges, power tunnel lining or dam body (or of a portion of it)
3 Earthquake	FM 3	Loss of freeboard or impairment of integrity of dam crest and spillway structure due to earthquake shaking
4 Structural failure of devices for water control (Middle Level Outlets, Spillway, Power Waterways, Power Houses, River diversion plug)	FM 4	Left or right abutments subject to uncontrolled leaks, developing major uncontrolled outflow or source of evident movements of the rock wedges, power or diversion tunnel linings or dam body (or of a portion of it). Middle Level Outlets structures subject to uncontrolled leaks or movements. Spillway structure subject to uncontrolled leaks or movements affecting the dam crest and/or its retention capacity. Reservoir level trend raising, predicted to exceed dam crest level. Structural failure of Intake tower(s). Blockage of power tunnel intake(s) opening, Obstruction of Power Waterways intake(s).
5 Gate failure/ power loss/ equipment failure/ fire	FM 5	Spillway or MLOs or Power Waterways gates unable to work in conjunction with possible flood events. Reservoir level trend raising, predicted to exceed dam crest level.
6 Excessive plunge pool scouring endangering dam foundations	FM 6	Dam foundation at downstream toe subject to uncontrolled scouring or displacements along main joints of the rock developing a major uncontrolled source of evident movements of the dam (or of a portion of it).
7 Extreme weather warning	FM 7	Reservoir level trend raising, predicted to exceed dam crest level.
8 Sabotage/ accident	FM 8	Attack that threatens the integrity of the dam or spillway or Power Waterways

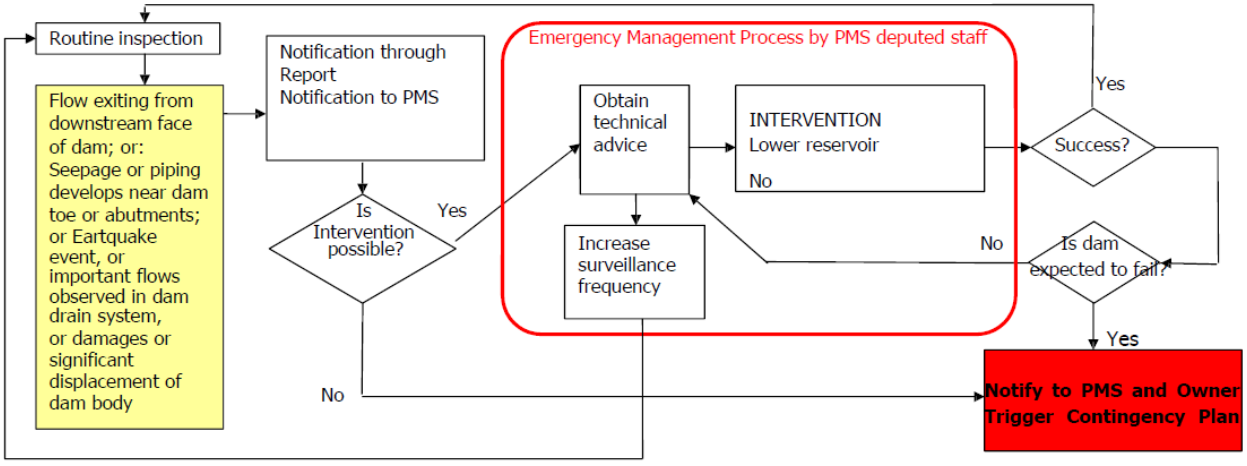
Table 34 – Failure modes for Action Plans

ACTION PLAN TYPICAL GRAPH

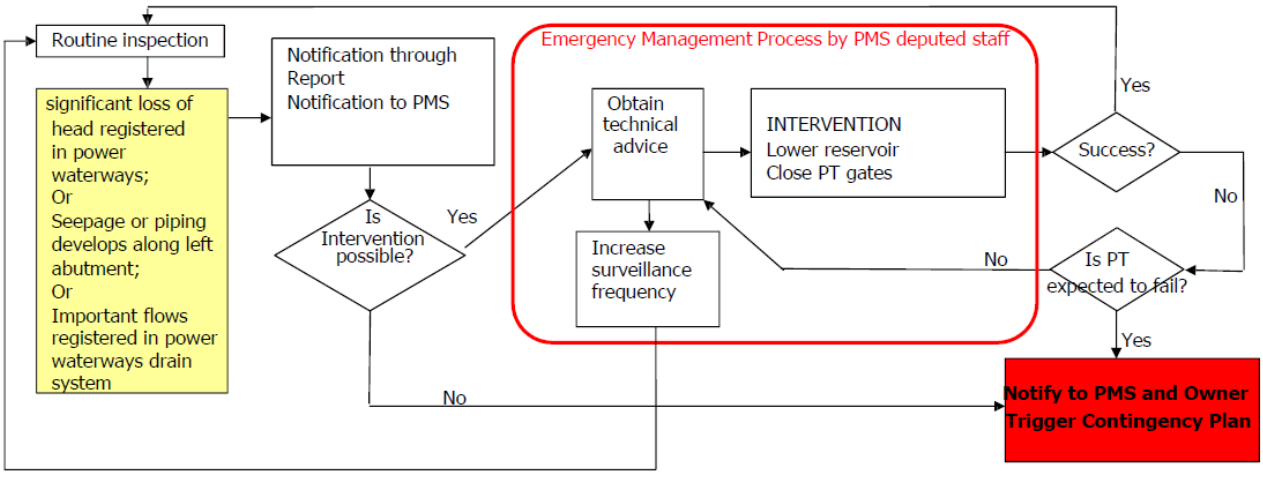


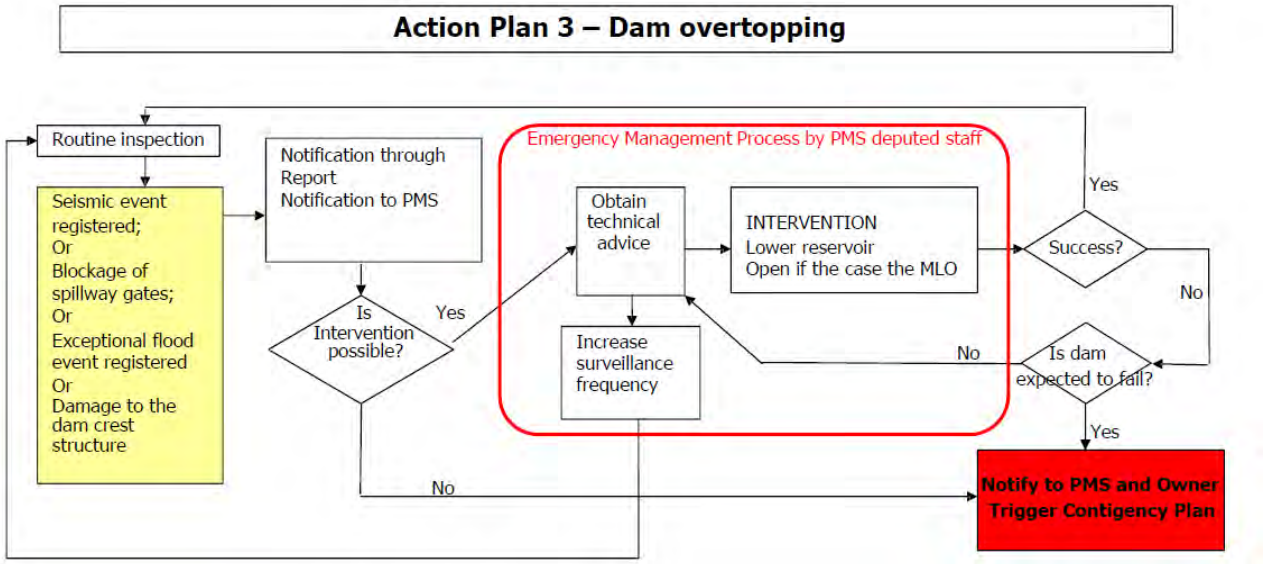
(1) The type and extent of the intervention shall be identified according to the case, taking into account the principles above indicated and in any case through the advice taken from an experienced dam safety engineer. In the next graphs the interventions for the main initiating events are shown. For Power Waterways and power house they are referred to the left abutment, but shall be considered as typical for both power plants.

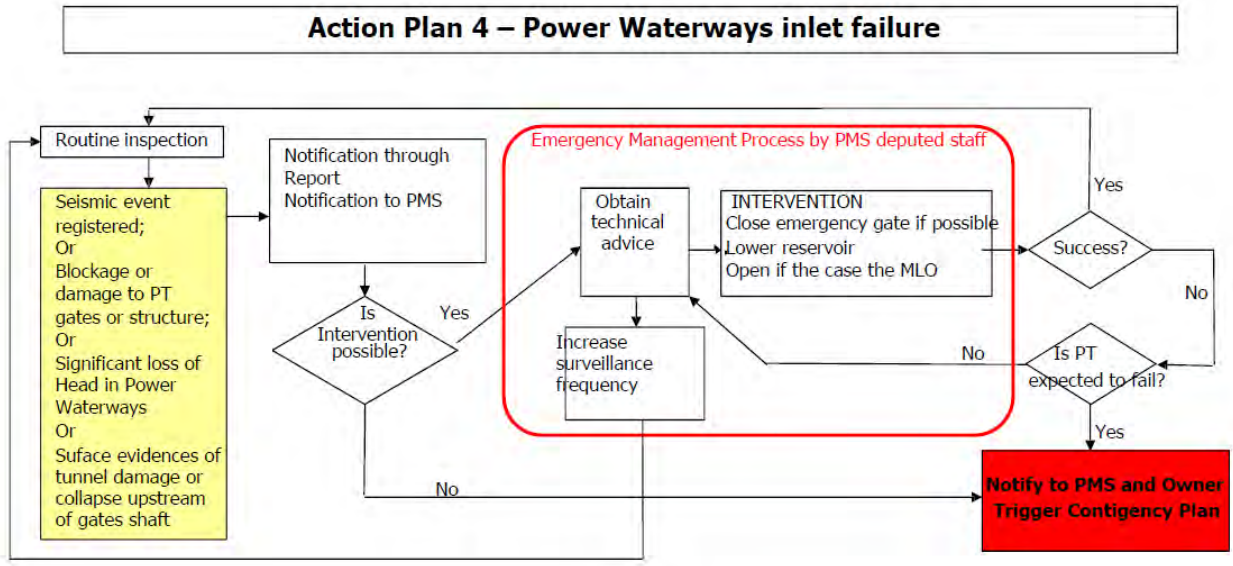
Action Plan 1 – Leaks or seepage through Dams, Foundations or Abutments

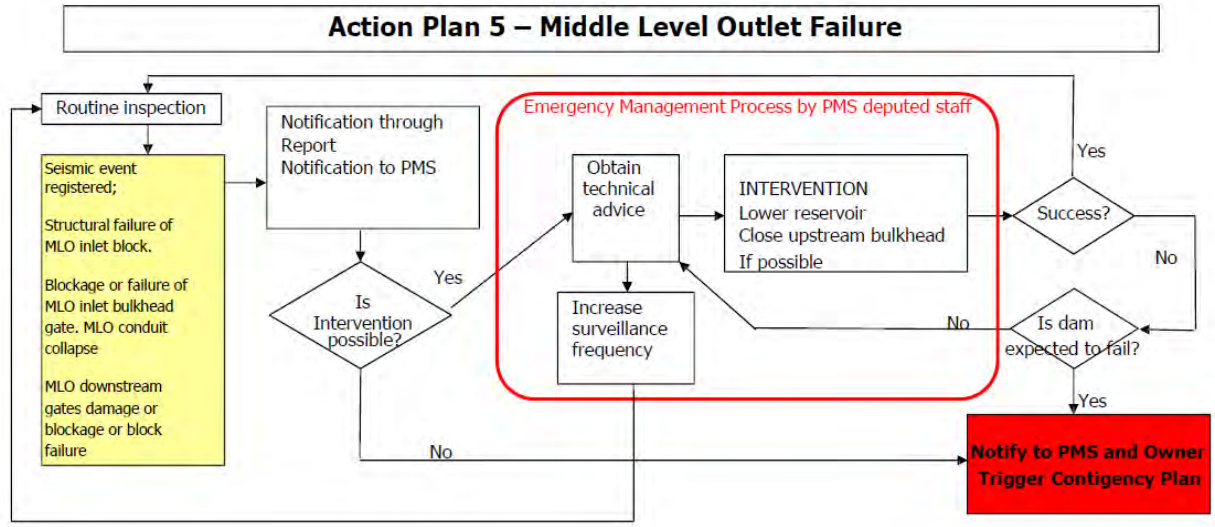


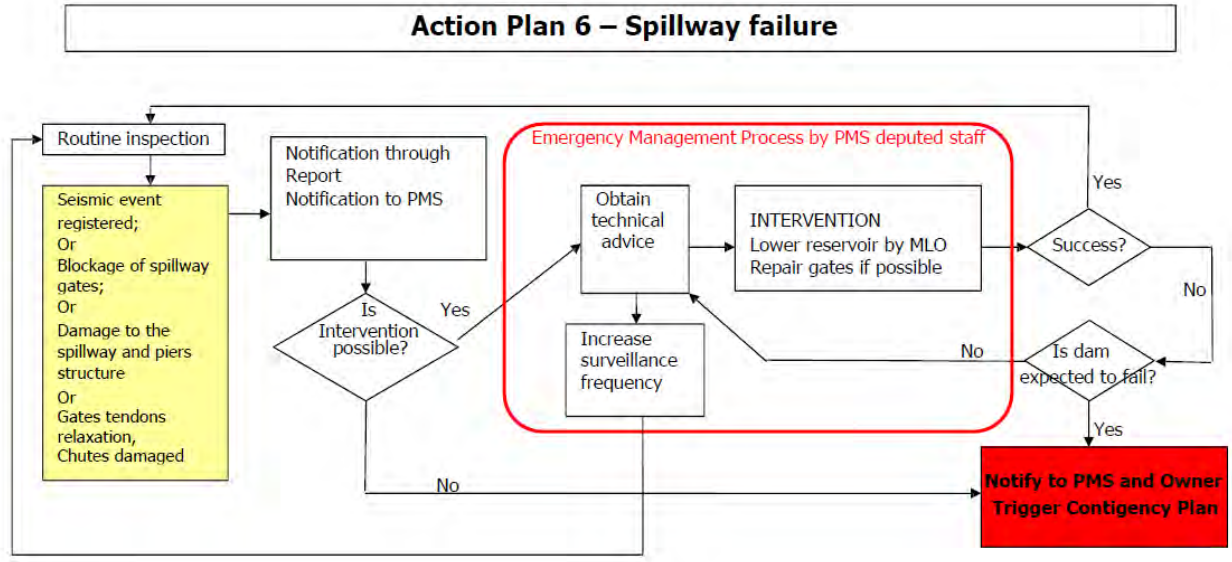
Action Plan 2 – Leaks of Power Waterways or seepage through left abutments

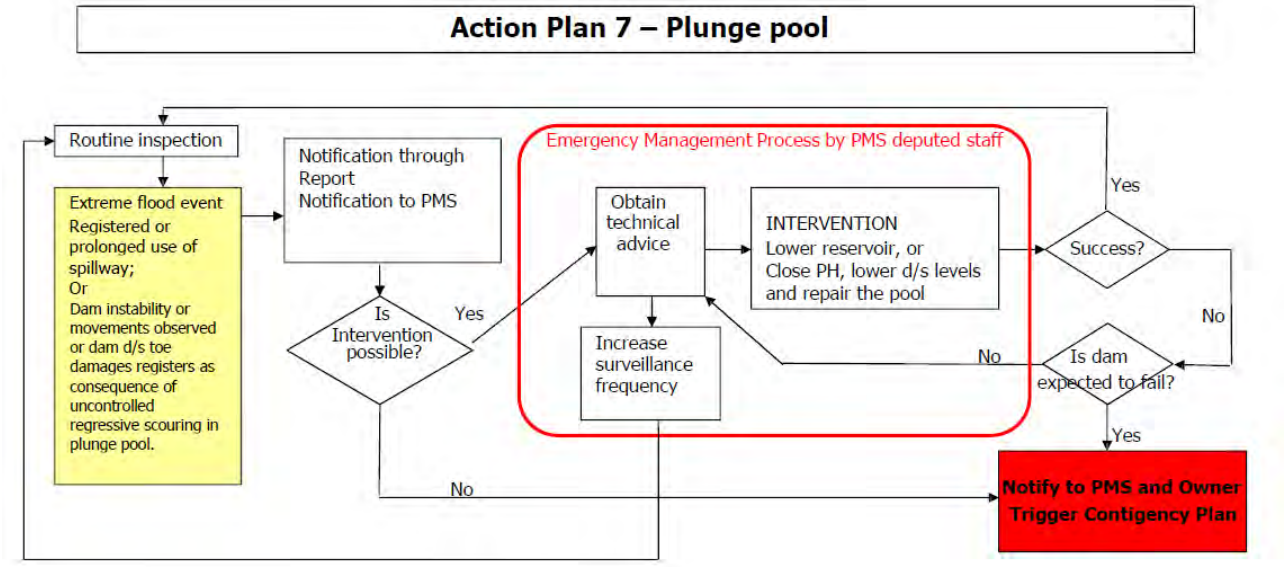


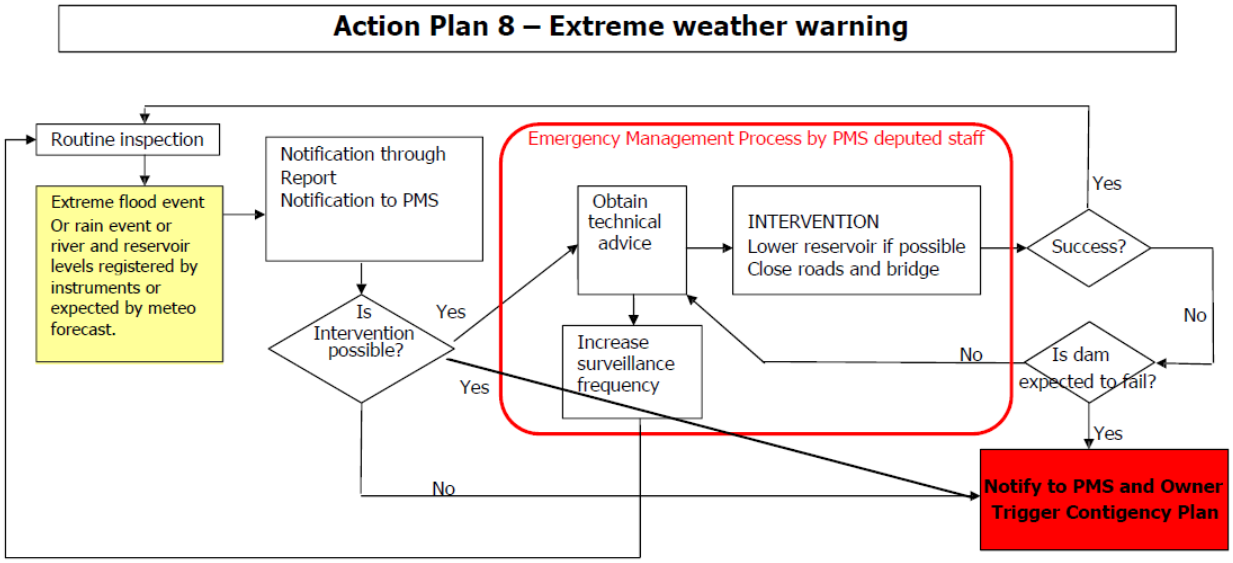

Action Plan 3 – Dam overtopping
Routine inspection
Seismic event registered;
Or
Blockage of spillway gates;
Or
Exceptional flood event registered
Or
Damage to the dam crest structure
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Lower reservoir
Open if the case the MLO
Increase surveillance frequency
Success?
Yes
No
Is dam expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan


Action Plan 4 – Power Waterways inlet failure
Routine inspection
Seismic event registered;
Or
Blockage or damage to PT gates or structure;
Or
Significant loss of Head in Power Waterways
Or
Suface evidences of tunnel damage or collapse upstream of gates shaft
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Close emergency gate if possible
Lower reservoir
Open if the case the MLO
Increase surveillance frequency
Success?
Yes
No
Is PT expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan


Action Plan 5 – Middle Level Outlet Failure
Routine inspection
Seismic event registered;
Structural failure of MLO inlet block.
Blockage or failure of MLO inlet bulkhead gate. MLO conduit collapse
MLO downstream gates damage or blockage or block failure
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Lower reservoir
Close upstream bulkhead
If possible
Increase surveillance frequency
Success?
Yes
No
Is dam expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan


Action Plan 6 – Spillway failure
Routine inspection
Seismic event registered;
Or
Blockage of spillway gates;
Or
Damage to the spillway and piers structure
Or
Gates tendons relaxation,
Chutes damaged
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Lower reservoir by MLO
Repair gates if possible
Increase surveillance frequency
Success?
Yes
No
Is dam expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan


Action Plan 7 – Plunge pool
Routine inspection
Extreme flood event
Registered or prolonged use of spillway;
Or
Dam instability or movements observed or dam d/s toe damages registers as consequence of uncontrolled regressive scouring in plunge pool.
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Lower reservoir, or
Close PH, lower d/s levels and repair the pool
Increase surveillance frequency
Success?
Yes
No
Is dam expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan

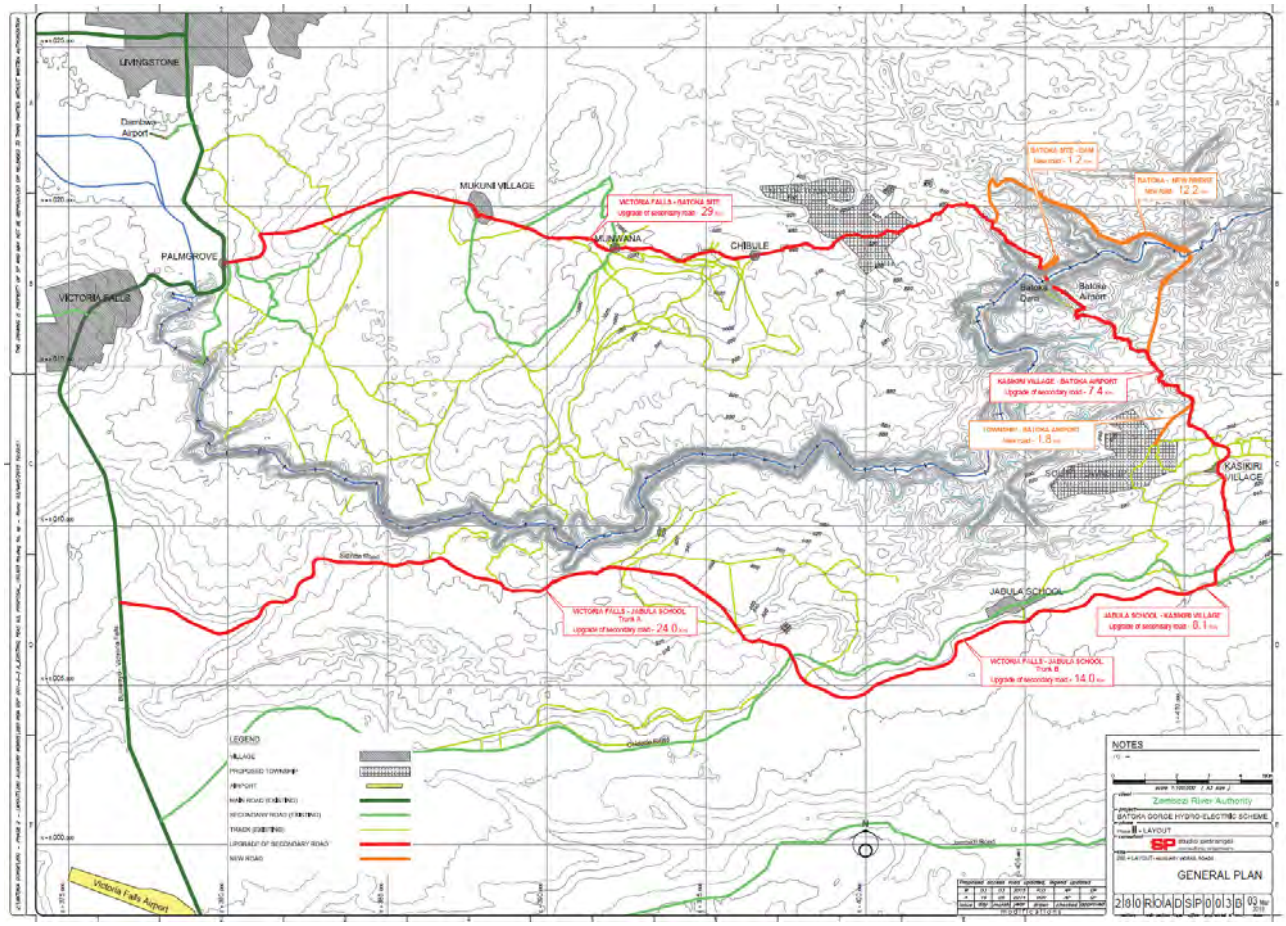

Action Plan 8 – Extreme weather warning
Routine inspection
Extreme flood event
Or rain event or river and reservoir levels registered by instruments or expected by meteo forecast.
Notification through Report
Notification to PMS
Is Intervention possible?
Yes
Yes
No
Emergency Management Process by PMS deputed staff
Obtain technical advice
INTERVENTION
Lower reservoir if possible
Close roads and bridge
Increase surveillance frequency
Success?
Yes
No
Is dam expected to fail?
Yes
No
Notify to PMS and Owner
Trigger Contigency Plan

E.6 PREPAREDNESS PLAN IMPLEMENTATION

E.6.1 ACCESS ROUTES – PRIMARY AND SECONDARY

Direct access roads to the Batoka dam and power houses are designed, at feasibility design level, coming from the nearest available roads and villages, as indicated in relevant drawings of the Feasibility Design (from which the next figure is extracted).

They are assumed to be developed and finalized at further detailed design stage.



A site map showing the location and main access routes to the dam, control buildings, Power House and other site structures shall be kept updated and attached to this RPP.

Access using bike or motorbike, or on foot, may be required under emergency circumstances if roads are impassable in usual vehicles. Access difficulties are likely if roads are damaged by earthquake or flood.

E.6.2 PUBLIC SAFETY

Public access to Batoka site (dam, power houses, switchyards) is assumed will be restricted using gates and fences as appropriate.
Access to the dam crest, intake tower and outlet areas will be restricted using gates and fences as appropriate.

E.6.3 SITE SECURITY

A security plan for the dam should be prepared and implemented by the PMS, considering the following aspects: dam size, location, hazard classification, importance to the economy, national defence and local public order.
In this regard, for the case of Batoka Plant, it shall be considered that:

- the dam size is very big,
- the location is impervious, but not extremely emote in respect to the main towns and roads of the country,
- the region downstream Batoka Plant is not uninhabited, but along the river it is not occupied by major townships,
- the plant is of great importance for the economy of the country.
- the plant has an international interest, being of transboundary nature and inserted in the network of neighbour countries.
- The plant is not far from Victoria Cascades, that is an international site attracting international visitors every year.

The plan shall identify the roles and responsibilities for site security and management of the reservoir area.

The following site areas should be in principle restricted and fenced off:

- Dam
- Dam abutments yard (in a zone of at least 100m beyond the abutments)
- Gate shafts intakes and relevant accesses
- Plunge Pool and relevant abutments, for a stretch of the river of at least 1km downstream.
- Surge shafts and relevant yards
- Accesses to access tunnels along the Power Waterways
- Power Houses (double fence shall be foreseen in the energized area of the HV transformers)
- Switchyards
- Control buildings and diesel generator buildings,
- River diversion (as far as applicable in consideration of the temporary nature of such work).

E.6.4 ON SITE RESOURCES

A security plan for the dam should be prepared and implemented by the PMS, considering the following aspects.
It is important to maintain sufficient resources on the site in order to swiftly handle emergencies if access to the dam is restricted. It is assumed that Batoka dam is continuously manned and that there are sufficient numbers of people living near the site to assist in carrying out emergency repairs.

Every incident is different and it is not possible to provide resources for every eventuality. The items listed in Table here below (that is indicative and can be integrated by PMS according to the need) are the basic resources that should be available to allow repairs to be carried out for the most likely incidents.

Early detection of emergencies can often restrict the emergency to an Internal Emergency and prevent it progressing to a Developing or Imminent Emergency. Therefore it is important to carry out routine dam safety surveillance by Batoka Project inspectors based at the dam.

Resources	*Location*
IMPORTANT EMERGENCY TOOLS	
Emergency communication system (such portable phones or radio)	
equipment Back-up power supplies	
Earthmoving equipment	
Drilling device for emergency small drain holes need	
Mobile crane	
Boat with engine	
BASIC EMERGENCY TOOLS	
Mobile floodlights and Torches	
First aid kit	
Welding Equipment	
Products for concrete crack sealing	

This list does not include ordinary spare parts assumed to be already available in the plant according to the project design.

Table 35 – Available emergency tools

E.6.5 CONTROLLED DRAWDOWN PROCEDURE

If deemed necessary, according to the analysis of the results of observations carried out during the surveillance inspections (as described in Operation and Maintenance Manual), the Controlled Drawdown Procedure or the Emergency Drawdown Procedure will be activated.

The Controlled Drawdown Procedure (CDP) is finalized to lowering the reservoir level without causing damages to the outlet structures such as Spillway, Power Waterways and Middle Outlets.

If according to the results of observations carried out during the Alert Procedure, an emergency state is triggered but any impending failure is excluded, the Controlled Drawdown Procedure will be activated to perform inspections and repair works on the submerged structures as necessary and as possible (no dewatering is physically possible below the operating range of the MLOs once reservoir is impounded).

This is of course valid only for impounding reservoir water levels above the operating range of the MLOs, and with the limits depending on the capacity of the available discharging devices versus the incoming flows (therefore also depending on the season and on the duration of the emergency state).

The reservoir drawdown is foreseen to inspect lower portions of dam upstream face, or the first portion of Power Tunnels upstream of the gate shafts.

The reservoir drawdown operations shall be carried out according to specific procedures that will be defined during the frame of the detailed design of the hydraulic structures.

The reservoir will be lowered, according to the water levels available in the reservoir within the minimum and maximum operating levels and to the Plant Operator choice, by means of:

- the Powerhouses units in operation (if at least one is maintained in operation 24 hours per day),
- the Ecological Discharge Device.
- the Spillway,
- the Middle Outlets.

E.6.6 EMERGENCY DRAWDOWN PROCEDURE

If the results of observations highlight an impending hazard for the dam safety, the Emergency Drawdown Procedure will be carried out in accordance to the instructions of the Contingency Plan described in the next chapter.

E.6.7 INFORMATION TO THE PEOPLE LIVING IN THE RESERVOIR AREA

- The population living in the zone of the impounding and downstream of the dam shall be informed about the reservoir regulation procedure, and in particular about the expected reservoir levels during normal and exceptional scenarios of operation (not only during the impounding period).

- Reservoir shore line will be marked on the ground in correspondence of roads, villages and other significant accessible points, with clear benchmarks or other evident signals on the shore line corresponding to the Maximum Operating Level, in order to give to the Owner(s) of the Plants and local Authorities a clear reference along the shoreline of the reservoir, for the operations of information and management of the ground that the Owner(s) will conduct.

- A program shall be implemented and provided by the PMS defining time and procedures required to inform the populations downstream of the Batoka dam of the artificial floods release whenever they are released from Middle Outlets and or from Spillway gates. This program will remain valid to be used during the current operation of the Plant, and updated in case it will be changed.
  Downstream areas shall be informed also in case of any sudden release of water is decided through the operation of the Batoka Hydraulic devices (Middle Outlets, Spillway, Power Waterways), and they shall be coordinated with the downstream plant of Kariba.

E.7 CONTINGENCY PLAN

E.7.1 INTRODUCTION

For the RCC dam the risk of failure is extremely unlikely. At the same time, sudden release of stored water is a risk that can occur for exceptional flood events or accidental manoeuvre of hydraulic devices (Spillway, Power Tunnels and MLOs gates).

E.7.2 IMPENDING FAILURE OR FAILURE

IMPENDING FAILURE

If, during the period of the impounding or during the operation of the plant, there is a suspect of impending failure, or phenomena that can compromise the safety of the main structures (Dam, Power Waterways and relevant Shafts, Power Houses or other main structures) or adjacent natural or excavated slopes, the following procedures must immediately be initiated:

1 Contact and inform the Project Head Offices.
Contacts are reported in the forms at the end of this Part. They shall be filled and maintained updated by the PMS.

2 Inspect and monitor the dam (or other interested) structure and hydraulic steel structures.

3 Determine additional immediate actions in accordance with the Project Management Structure offices to be taken to reduce the risk of failure and any other necessary further actions.

4 In case it is decided to proceed with the partial drawdown of the reservoir, and if and if there are not the conditions to implement a controlled drawdown procedure described in this report at par. E.6.5 CONTROLLED DRAWDOWN PROCEDURE, coordinate efforts with the Owner and the Local Authorities in alerting all downstream areas of the rapid increase of releases (procedures for Large or sudden releases downstream of the dam).

FAILURE

If the Dam, or part of it, is failing, notice shall be immediately given by radio or by phone to the Owner(s) Head Office (see contacts at the end of this report), and simultaneously to Local Authorities (local police and municipalities, emergency evacuation units or mobile alarm team to be sent on the critical zones) that will take actions to trigger alarm and evacuation plans on the downstream areas, considering the areas susceptible of inundations as individuated in Dam Break Analysis document (see following chapter), whose final version is

assumed will be prepared in detailed design stage and relevant maps included in this volume for easy of consultation.

The detailed procedure for implementation of this notice shall be developed by the PMS. It shall be developed considering the transboundary nature of the Project, the structure of Ownership and Operation that will have this plant, as well as the need to coordinate any important decision about water management with the downstream plant of Kariba.

E.7.3 LARGE OR SUDDEN RELEASE DOWNSTREAM OF THE DAM

EMERGENCY DRAWDOWN

In case a large water release downstream of the dam is imminent due to the actuation of the Emergency Drawdown of the reservoir the following procedure shall be followed:

1 the people working on site (if any) shall be immediately informed, and the working areas downstream of the dam close to the river evacuated;
2 trigger the "Large water release" alarm;
3 Employer and Local Authorities that control the critical zones shall be immediately contacted (see contacts at the end of this report) and informed.

ACCIDENTAL MANOEUVRE OF THE OUTLET GATES

In case of large and sudden water releases due to an accidental manoeuvre of the radial gates of the spillway, which cannot be closed within 15 minutes, the "Large water release" alarm shall be triggered and the Operator and Owner's Head Office informed (see contacts in the last paragraph of this report).

"LARGE WATER RELEASE" ALARM

The alarm is used in case of:

- Opening of the radial outlet gate during exceptional flooding.
- Opening of the Spillway and Middle Outlet gates for Reservoir Emergency Drawdown.
- Accidental opening of the spillway gates (according to procedure in previous paragraph).
- Impending danger of overtopping: the alarm is risen on the basis of the water elevation in the reservoir. Such event is extremely unlikely..

The alarm shall be triggered in order to assure at least 6 hour for the evacuation of the people working on site.

The alarm is given through notice the PMS and Owner Head Offices (see contacts reported in last paragraph of this report), that in turn will contact the Local Authorities (local police and municipalities, emergency evacuation units or mobile alarm team to be sent on the critical zones) to take immediate action in alerting populations and managing the emergency in the critical zones.

E.7.4 CONTACTS FOR EMERGENCY or ALARM CASES

In case of need to trigger an ALARM condition, or in case of need to implement an exceptional operation described in this part of the report, the addresses reported here below shall be immediately contacted:

Plant Management Structure				
Name	Position	Telephone	E-mail	Mail address

Local/Governmental Authorities				
Name	Position	Telephone	E-mail	Mail address

Table 36 – Contacts for Emergency or Alarm cases

E.8 DAM BREAK ANALYSIS

E.8.1 INTRODUCTION

The present chapter describes the results of the hydraulic calculations carried out to identify the flooded areas in case of emergency.

The Terms of Reference of the assignment at paragraph A.3.3.2 states that:
The Consultant shall carry out one-dimensional dam-break analysis and assess the downstream impact of such failure. The results of the dambreak analysis will be used to prepare inundation maps …".

Purpose of this chapter is to provide the inundation maps as a result of the hydraulic calculation carried out for the dam break analysis.

In detail, the evaluation of the flooded areas located downstream of the Batoka dam has been carried out accordingly to the following scenario:

- DAM BREAK - Incoming of an extreme climate event meteorological event, which causes dam overtopping. This scenario has been evaluates on the base of the following foresees assumptions:
    - incoming of the PMF hydrograph while the reservoir is at full supply level (w.l. = 757 m asl);
    - flap gates blocked in the closed position (i.e. raised and retaining the upstream reservoir at OL max);
    - raising of the reservoir up to overtopping of the dam crest (crest level = 762 m asl);
    - almost instantaneous breach formation with consequent break of the entire dam.

This scenario assumes catastrophic characteristics and, as it will be further explained, capable of producing a flood event with peak discharge equal to approximately 803'870 m3/s.

Hydraulic analysis, which has been carried out using the HEC-RAS program, allowed to identify areas subject to flooding and to determine the hydraulic levels reached in the downstream areas of the project, the average speed of the water as well as the travel time of flood peak.

The study of the flood propagation has been analyzed for a river stretch of approximately 120 km length, along the Zambezi river, from the Batoka dam to the Lake Kariba.

The inundation maps that show the results of the dam break analysis are illustrated in the following annexed drawings:

- 346 DBK D SP 001 Inundation area, General, 300k
- 346 DBK D SP 002 Inundation area, Key map, 300k
- 346 DBK D SP 003 Inundation area, plan 100k , sheet 1 of 4

- 346 DBK D SP 004 Inundation area, plan 100k , sheet 2 of 4
- 346 DBK D SP 005 Inundation area, plan 100k , sheet 3 of 4
- 346 DBK D SP 006 Inundation area, plan 100k , sheet 4 of 4

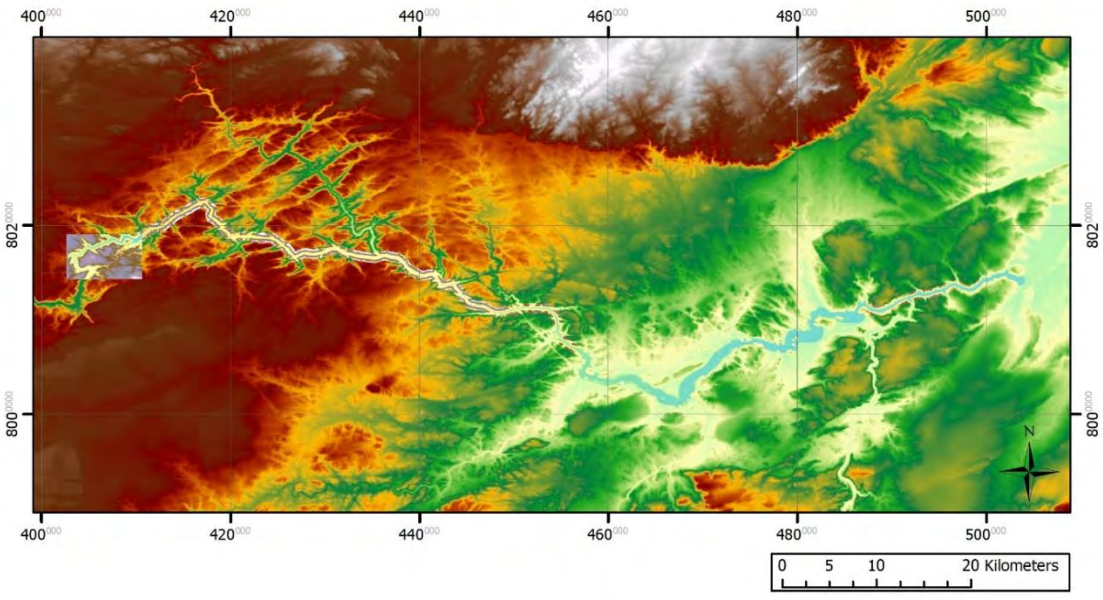
E.8.2 TOPOGRAPHIC DATA

Modeling of water propagation over complex topographical surfaces for floodway delineation requires an accurate and reliable topographical database; for this report's aim, the following digital elevation models of the terrain were used:

- LIDAR (LASER SCANNING) with an resolution of 0.3 m;
- SRTM (Shuttle Radar Topography Mission) with an resolution of 30 m;

The LIDAR DTM is the most accurate among the two adopted digital models, and covers the main Zambezi river channel and close surroundings. In order to allow a complete floodplain delineation the LIDAR has been joined with the SRTM DEM, following the FEMA guidelines.

The following figure show the topographic database used for the hydraulic calculations:



DIGITAL ELEVATION MODEL (WGS 84/UTM zone 35S)

E.8.3 VALLEY MORPHOLOGY AND HYDRAULIC CHARACTERISTICS OF STREAMFLOW

The study of the flood propagation has been analyzed for a river stretch of approximately 120 km length, along the Zambezi river, from the Batoka dam to the Lake Kariba

Downstream of the dam, the river valley is particularly engraved, with both side of the valley very steep.
The entire river presents a pronounced slope, in particular the river longitudinal slope can be summarized as follows:

- 0.35 % from 0 to about 8.2 km ds of the dam;
- 0.17 % from 8.2 km to about 35 km ds of the dam;
- 0.1 % from 35 km to about 76 km ds of the dam;
- 0.006 % from 76 km to about 120 km ds of the dam.

E.8.4 FLOOD ROUTING MODEL - HEC RAS version 5.05 (2D)

Dam Break modelling consists in:

- Prediction of the outflow hydrograph at the dam due dam breach;
- Routing of hydrograph through downstream valley in order to obtain the maximum water level and discharge, along with the time of travel at different locations of the river downstream of the dam.

The HEC-RAS program, developed by the US Army Corps of Engineers (USACE) was used for dynamic flood routing and dam safety analysis. The HECRAS software is based upon a highly optimized version of the National Weather Service (NWS) 1988 dam-break flood forecasting program DAMBREAK.
HEC-RAS is used to hydraulically route the flood through the river and downstream valley.

The two dimensional (2D) model is considered more suitable than the one dimensional or quasi two dimensional model to analyze the routing of the flood wave generates due to dam failure.
Indeed, when a flood wave enters an unconfined floodplain, a two dimensional model is appropriate to simulate the behavior of the flow and to simulate the propagation of the wave since floodwater generally presents frequent divergence of flow in the plain.

The governing equation of the model is the two-dimensional flow de Saint Venant's equations, referred also as Shallow Water equations.
The Swallow Water Equations are adopted when:

- The horizontal scale lengths are much larger than the vertical scale lengths ; and
- Vertical pressure gradient is almost hydrostatic.

In case of propagation of the wave in an unconfined floodplain, the flow is shallow enough and since the vertical accelerations can be neglected, then it can be shown that a good approximation to the flow is to replace all flow variables by their averages in vertical direction. The resulting, depth-averaged, three-dimensional equations of motion then become a two-dimensional representation in the horizontal plane.

In particular, the unsteady differential form of the Mass Conservation (continuity) and Momentum Conservation, considering the hypothesis of the shallow water, recalled above, are shown below:

$$\frac{\partial H}{\partial t} + \frac{\partial (uh)}{\partial x} + \frac{\partial (vh)}{\partial y} + q = 0$$

$$\frac{\partial u}{\partial t} + u\frac{\partial u}{\partial x} + v\frac{\partial u}{\partial y} = -g\frac{\partial H}{\partial x} + v_t\left(\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2}\right) - c_f u + fv$$

$$\frac{\partial v}{\partial t} + v\frac{\partial v}{\partial x} + v\frac{\partial v}{\partial y} = -g\frac{\partial H}{\partial y} + v_t\left(\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2}\right) - c_f v + fu$$

Where t is time, H is a water surface elevation, h is the water depth, q is a source or sink term, u and v are the velocity components in the X and Y direction, g is the gravitational acceleration, vt is the horizontal eddy viscosity coefficient, cf is the bottom friction coefficient and f is the Coriolis parameter. (Chaudhry, 2008, Brunner, 2016).

The first term in the moment equation represents the local acceleration, the second term is the convective acceleration, and the further terms describe the forcing from gravity, eddy viscosity, bed friction, and Coriolis force. Using the Manning's formula, the friction coefficient cf can be expressed as following (in the x-direction):

$$c_f = \frac{n^2 g|u|}{R^{4/3}}$$

Where n is Manning's coefficient, g the gravitational constant, u the velocity in the x-direction and R the hydraulic radius.

The HEC RAS solver uses, for the 2D unsteady flow equation, an Implicit Finite Volume algorithm which provides an increment of improved stability and robustness over traditional finite difference and finite element techniques. With this kind of algorithm, Hec-Ras can handle a completely dry 2D element during a sudden rush of water, which is typical in a dam break analysis; furthermore the algorithm can perform subcritical, supercritical and mixed flow regimes.

In order to implement the numerical model the following steps were to:

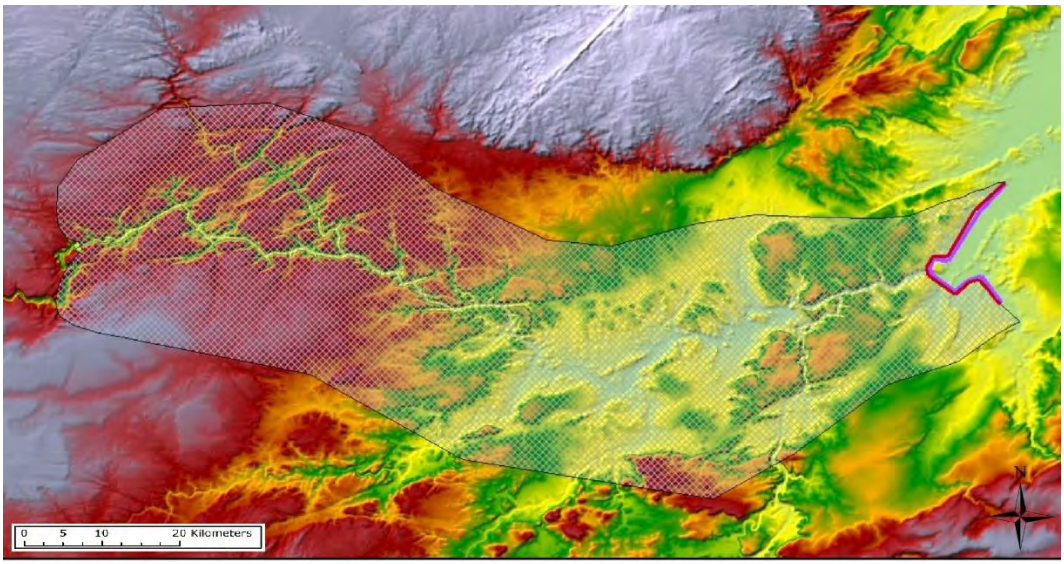
- develop the geometry and therefore the computational mesh.;
- define the roughness coefficient based on the land cover map;
- evaluate boundary and initial condition.

E.8.5 MODEL GEOMETRY – COMPUTATION MESH

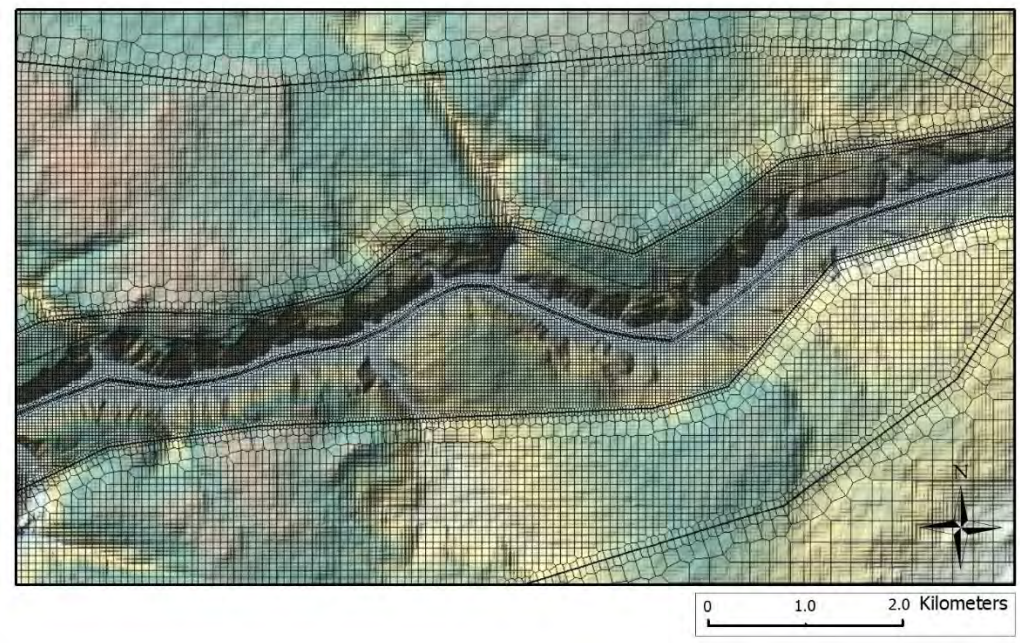
A georeferenced HEC-RAS computation model was prepared, by taking advantage of the HEC-RAS mapper utility.

The model consists in an unstructured computational mesh spreading from the dam toe to the Kariba Lake.

The main river and creek paths were used as break lines in order to align the mesh faces to water courses. In the same way, any kind of barriers to the flow were highlighted and introduced in the construction of the computational mesh by use of break lines.



EXTENSION OF THE COMPUTATION DOMAIN



DETAIL OF COMPUTATIONAL MESCH

Considering a square geometry of the cells, the main characteristics of the domain geometry for this analysis are listed below:

- Minimum cell size 10x30 m (in correspondence of the river bed)
- Maximum cell size 200x200 m (external of the inundation area)
- Total cell numbers 634’000

The cells' dimensions are smaller where needed, for example along the path of the watercourses, in order to correctly describe the thalweg, or where the slope of the terrain is higher.
The size of the grid cells defines the resolution of the model and model results. For this reason, a sensitivity analysis was carried out in order to define the optimum mesh size to be used for these preliminary simulations. Identifying the optimal mesh size is an important activity for a successful numerical modelling. This is a key parameter since a smaller mesh requires a higher computational running time, on the other hand, a larger mesh might reduce the accuracy and/or the stability of the model

E.8.6 LAND COVER and MANNING'S COEFFICIENTS

The Manning n flow resistance figures for the riverbed and banks, have been determined using the VEN TE CHOW method, as quoted in OPEN CHANNEL HYDRAULICS (1959). In applying the above method, a morphological/vegetation observed on the site survey, has been correlated to the reference table.

Considering that the n value of the flood plain is generally larger than that of the channel, SP has assumed two different value for Manning's coefficient, in particular:

- Major stream (regular section with no boulders or brush) n = 0.025;
- Flood plain (scattered brush, heavy weeds) n = 0.05

E.8.7 BREACH HYDROGRAPH PREDICTION AND BOUNDARY CONDITIONS

Two boundary conditions were applied, geographically defined by 2 ESRI shape files;

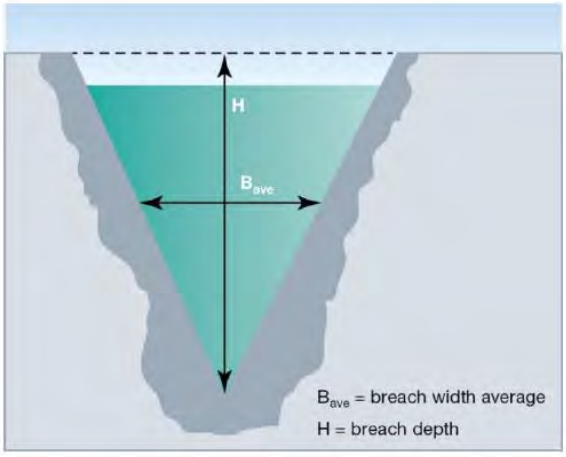
- Upstream condition;
    - Breach hydrograph with a flow peak equal to 803'870 m^3/s
- Downstream condition
    - full supply level at Kariba Lake, equal to 488.5 m asl

The inflow condition was applied at the upstream limit of the mesh (in correspondence of the dam toe) and the downstream boundary condition was applied in correspondence of the Kariba Lake.

The upstream condition is a critical point in the dam break analysis. Predicting the outflow hydrograph is strongly related to the prediction the breach characteristics, which include the parameters needed to physically describe the breach (breach depth, breach width, and side slope angles) as well as parameters that define the time required for breach initiation and development.

According the FEMA guideline, for concrete arch dam, the breach widths range from 80 percentage of the entire length of the dam to the entire length of the dam and the breach side slope is assumed to range from vertical to the slope of the valley wall. The breach formation time for modelling purposes ranges from instantaneous to 0.1 hours (USACE, 1980 and 2007; FERC, 1988; Fread, 2006).

The following imagine illustrates the schematic breach of the concrete arch dam



Schematic breach geometry progression of a concrete arch dam

Following the FEMA guidelines, for sake safety, SP has assumed an almost instantaneous removal of entire dam (0.1 h time is the time of fully breach formation).

In this case, the peak breach hydrograph has been computed used the Ritter's formula which represent a simplified of the De Saint Venant equation (USACE, 97)

$$Q_p = B_{ave} * \frac{8}{27} h_0 \sqrt{g h_0}$$

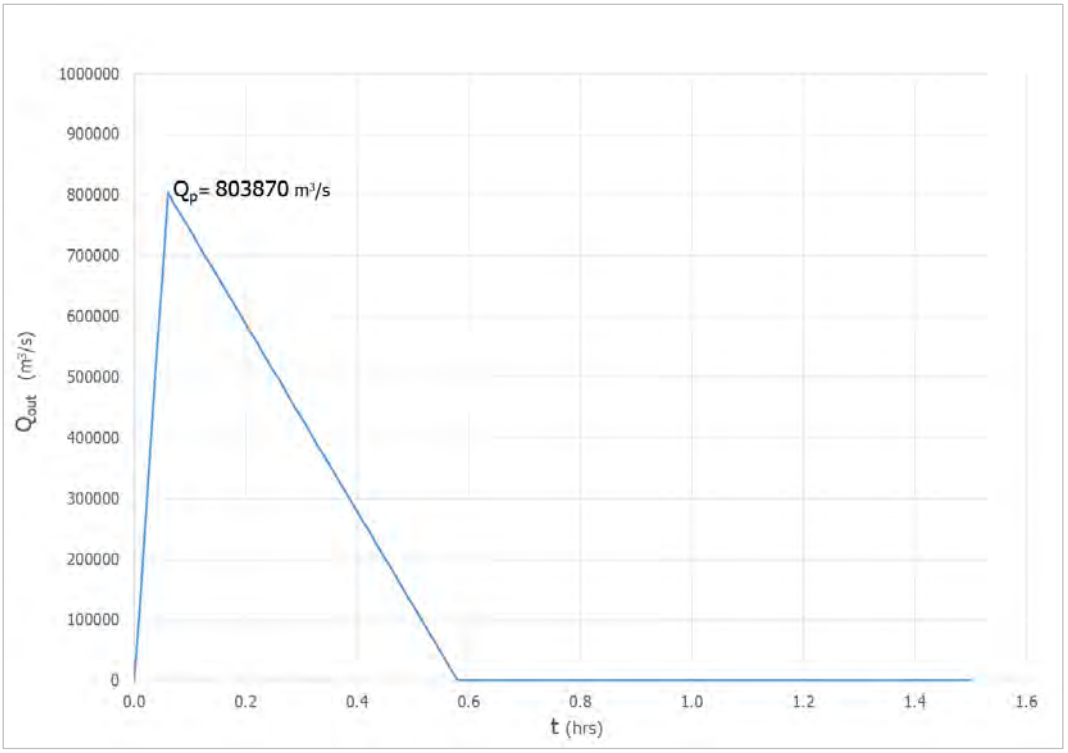
Where:

- Q_p is the peak breach flood;
- B_{ave} is the average width of the breach, which correspond with the average length of the entire dam;
- h_0 is the initial water depth at the dam, which is assumed equal to the dam height;
- g is the acceleration of gravity

The following data has been considered:

- Dam height = 160 m
- B_{ave} = 428 m
- Peak flow = 803'870 m^3/s

The beach hydrograph, illustrated in the graph below, has been obtained considering the fully emptying of the reservoir after the failure of the entire dam due the overtopping.



Dam Break hydrograph – Upstream boundary condition

The downstream boundary condition has been set as a constant stage hydrograph equal to 488.5 m asl, corresponded at the Full supply level of the Kariba Lake.

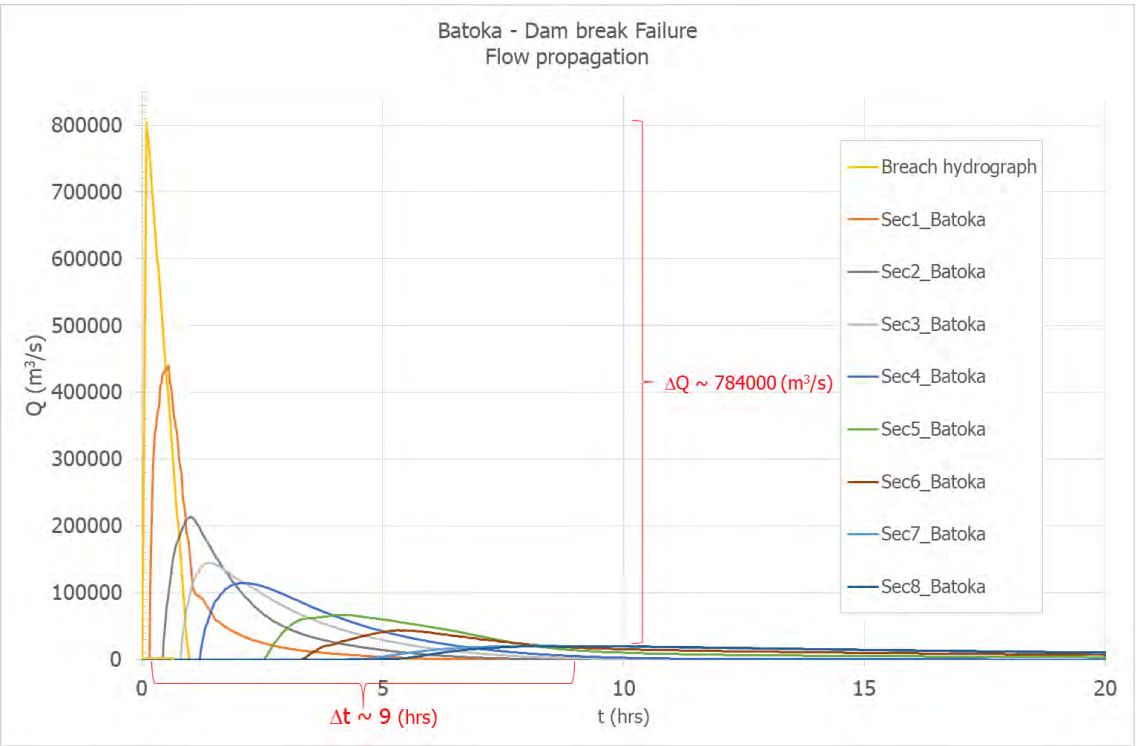
E.8.8 RESULTS

The main results of the hydraulic model for the dam break of the Batoka dam due overtopping re detailed in the following enclosed drawings:

- 346 DBK D SP 001 Inundation Area, General Plan, 300 k
- 346 DBK D SP 002 Inundation Area, Key Map, 300 k
- 346 DBK D SP 003 Inundation area, plan, 100 k, sheet 1 of 4
- 346 DBK D SP 004 Inundation area, plan, 100 k, sheet 2 of 4
- 346 DBK D SP 005 Inundation area, plan, 100 k, sheet 3 of 4
- 346 DBK D SP 006 Inundation area, plan, 100 k, sheet 4 of 4

The graphs below illustrate:

- The discharge's trend in the time for different section downstream of the Batoka dam;
- Inflow to the Kariba Lake;
- The maximum flow at each section with relative arrival time.



Dam Break hydrograph propagation

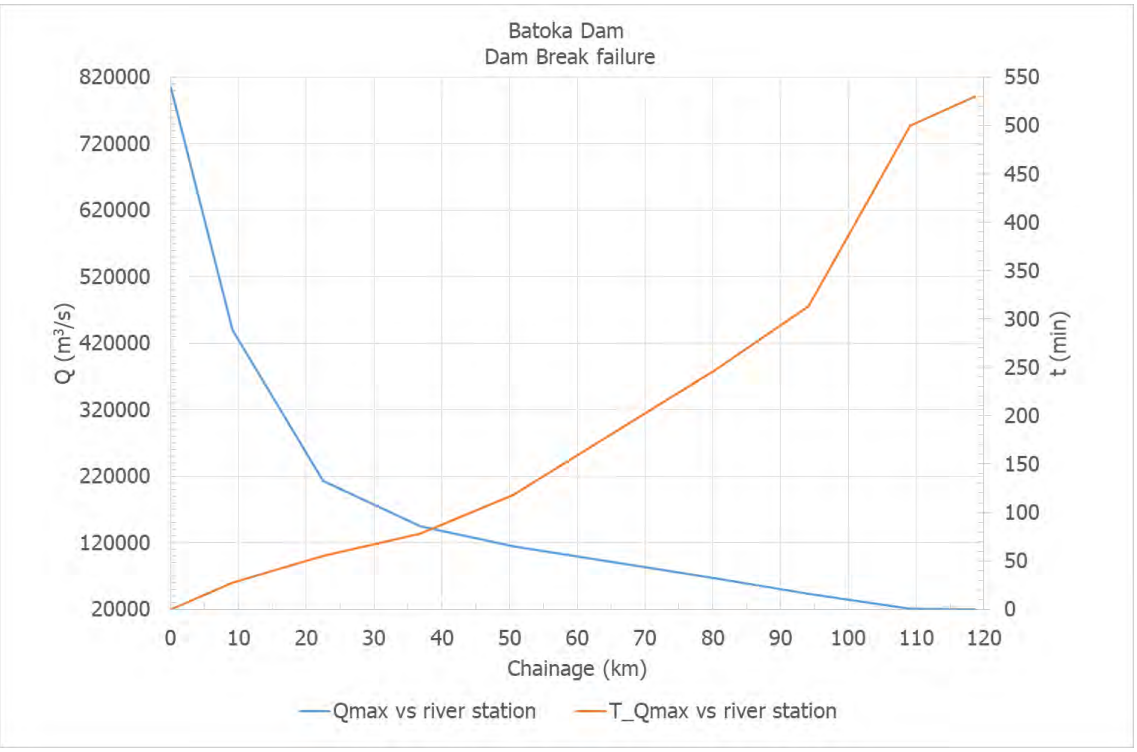
As far as the flow propagation is concerned, the peak of flow goes from 803'870 m^3/s at the dam toe, to 20'000 m^3/s at the last cross section before Kariba reservoir (sec 8), with a total reduction of 784'000 m^3/s.

Downstream of Batoka Reservoir, along the Zambezi River, the flow reaches Kariba reservoir, whose capacity is way larger than the one of Batoka reservoir, which is approximately equal to 1'400 Mm^3 at dam crest. Indeed, according the data recalled in the Main Report of Sinohydro (2005), the volume of the Kariba reservoir is approximately:

- 54 Mm^3 at 475.5 m asl (corresponding with the minimum reservoir level);
- 64'798 Mm^3 at 488.5 m asl (corresponding with the maximum reservoir level);

Being very large than upstream reservoir, Kariba reservoir allows the full attenuation the incoming flood.

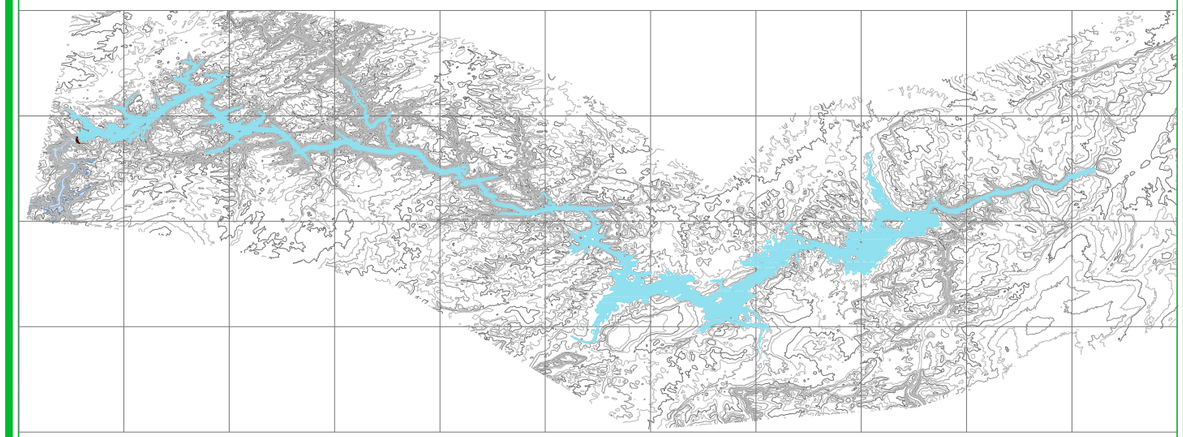
The arrival time of the peak, which is the difference in time of the peak at the dam toe and the peak of the flow hydrograph simulated in the last downstream cross section, is about 9 hours.



Propagation of the peak flow

Zambezi River Authority

BATOKA GORGE HYDRO-ELECTRIC SCHEME



Phase III - FEASIBILITY

Vol. 7 - DAM SAFETY PLAN

Annex A - Drawings

May 2019

Number												
3	4	6	G	E	N	R	S	P	0	0	1	A	May 2019
section		subsection		type	office	serial n°			rev.	date		

studio pietrangeli consulting engineers

PHASE III - FEASIBILITY

346 - DAM SAFETY

DBK - DAM BREAK

CODE						DATE	TITLE
Section	Subsect	Type	Office	Number	Rev.		
346	DBK	D	SP	001	A	15-mag-19	Inundation area, General, 300k
346	DBK	D	SP	002	A	15-mag-19	Inundation area, Key map, 300k
346	DBK	D	SP	003	A	15-mag-19	Inundation area, plan 100k , sheet 1 of 4
346	DBK	D	SP	004	A	15-mag-19	Inundation area, plan 100k , sheet 2 of 4
346	DBK	D	SP	005	A	15-mag-19	Inundation area, plan 100k , sheet 3 of 4
346	DBK	D	SP	006	A	15-mag-19	Inundation area, plan 100k , sheet 4 of 4

The above noted four drawings are not included due to the size of the drawings. To request a copy please contact ERM batokagorgehes@erm.com